PROPOSED INTEGRATED HEALTH & EDUCATION PRECINCT DESIGNATION

58-68 DELANCEY STREET, ORMISTON

Traffic Impact Assessment For 'The Hub Precinct Pty Ltd'



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1. INTRODUCTION

Lambert & Rehbein (SEQ) Pty Ltd has been commissioned by The Hub Precinct Pty Ltd to undertake a Traffic Impact Assessment for a proposed mixed use commercial development located at 58-68 Delancey Street, Ormiston QLD 4160, which is formally described as 0/SP308738, 0-2/SP308739; 0/SP308740; 4/SP308740; and 10-16/SP314782. The development site currently contains existing and approved medical and education buildings, a car park, and a portion of vacant land, with a total site area of approximately 5.2ha.

The proposed designation is for an Integrated Health & Education Precinct, consisting of a private hospital, medical consulting suites, research and education facility, residential aged care, and ancillary retail." This report has considered the sites full development potential which also includes childcare, independent living and a community hub which will be assessed at a future date via the DA process. It is understood that the development will be completed over three (3) stages. The proposed development proposes to gain access to the external road network via a new signalised intersection on Finucane Road and an existing access on Delancey Street. The proposed site layout, prepared by Destravis Group, is attached in **Appendix A**.

The development application is being pursued through the Ministerial Designation process and this traffic assessment forms part of the application material. We note that a previous detailed traffic assessment had been undertaken focussing on the proposed access to Finucane Road and was documented in a Lambert & Rehbein Technical Note (B19590TN002 Rev C) dated 10/11/2021. The intent of this initial separate engagement was to review the proposed new site access and demonstrate that a signalised intersection could be implemented in this location without significant implication for safety and efficiency of the state-controlled road network.

Subsequent to this previous work we have undertaken further detailed traffic analysis with a more networkbased traffic analysis that has been undertaken in accordance with the requirements of the GTIA. This report documents the work undertaken and the findings and recommendations.

The site will be referred to as the 'development site' from hereon in.

This report has been undertaken to assess the potential impact that the proposed development could have on the external road network surrounding the site, and is set out as follows:

Section 2 discusses the existing land use and traffic arrangements in the vicinity of the proposed development site.

Section 3 provides details of the proposed development site, including an assessment of the site layout and access arrangements.

Section 4 displays the calculations and assumptions used to establish the forecast generation and distribution of the proposed development traffic including a network aggregate delay assessment based on Microsim Modelling.

Section 5 summarises the key outcomes of the traffic investigations.

Lambert & Rehbein has derived the data in this report primarily from the data provided by the Client, and a desktop site investigation.

This report has been prepared on behalf of and for the exclusive use of the Client, and is subject to and issued in connection with the provisions of the agreement between Lambert & Rehbein and the Client. Lambert & Rehbein accepts no liability or responsibility whatsoever for or in respect of any use of or reliance upon this report by any third party.

2. CONTEXT OF THE DEVELOPMENT SITE

This section of the report describes the context of the proposed development and includes a description of the existing road network, adjacent land uses, and existing public transport facilities servicing the site.

2.1 DEVELOPMENT SITE

The development site is located at 58-68 Delancey Street, Ormiston QLD 4160, which is formally described as 0/SP308738, 0-2/SP308739; 0/SP308740; 4/SP308740; and 10-16/SP314782. The development site currently contains existing and approved medical and education buildings, a car park, and a portion of vacant land, with a total site area of approximately 5.2ha.

The development site is zoned partially as 'Low Impact Industry' and partially 'Recreation and Open Space' as per the Redlands City Council Planning Scheme. The development site is generally surrounded by 'Low Density residential' developments to the north, 'Community Facilities' to the south and 'Medium Density residential' and 'neighbourhood centre' developments to the east. The development site in the context of the external road network is shown in **Figure 2-1**. The subject site has southern frontage to Finucane Road and eastern frontage to Delancey Street. There are no other road frontages from which can gain access.



Figure 2-1 Proposed Development Site

2.2 EXISTING ROAD NETWORK

Inspection of the land use, road condition, intersection characteristics, public transport facilities, pedestrian access, and cyclist provisions surrounding the proposed development site has been undertaken in preparation of this assessment. This was completed to collect information about the road network operation, safety characteristics, public transport network and specific network / land-use factors potentially of influence to the proposed development.

2.2.1 FINUCANE ROAD

Finucane Road runs along the southern frontage of the development site. Finucane Road is gazetted as a 'State Controlled Road' under the jurisdiction of the Department of Transport and Main Roads (DTMR). Additionally, we note that this section of Finucane Road at the site frontage is gazetted as a 'Limited Access Road' (LAR 2) and has a defined limited access policy. This is discussed further in **Section 3** following.

The general form of Finucane Road is shown in **Figure 2-2** and was observed to have the following characteristics at the site frontage:

- Two-way, four-lane, median divided road;
- Kerb and channel exist on the eastbound side of the road (approx. 260m along the site frontage);
- Carriageway width of approximately 16.22m (including approx. 2m median) at the site frontage;
- Finucane Road meets Delancey Street at a four-way signalised intersection, more details in **Section 2.2.3**;
- No parking permitted on either side of the road;
- Pedestrian footpaths provided on the eastbound side of the road along the site frontage; and
- Posted speed limit of 70 km/hr.



Figure 2-2 Finucane Road (Facing west)

2.2.2 DELANCEY STREET

Delancey Street is located on the eastern frontage of the development site. Delancey Street is gazetted as a 'local road' as per Redlands City Council Planning Scheme. The general form of Delancey Street is shown in **Figure 2-3** and was observed to have the following characteristics at the site frontage:

- Two-way, two-lane, median divided road;
- Kerb and channel exist on both sides of the road;
- Carriageway width of approximately 17.6m (including median and turn lanes as can been seen in **Figure 2-3**) at the site frontage;
- Delancey Street meets Finucane Road at a four-way signalised intersection, more details in **Section 2.2.3**;
- Shoulder parking permitted on both sides of the road;
- Pedestrian footpaths provided on both sides of the road; and
- Posted speed limit of 50 km/hr.

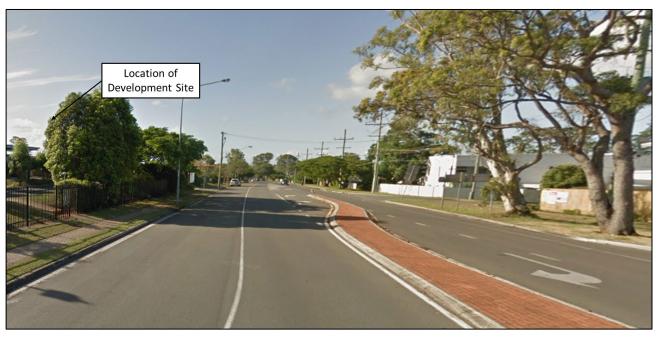


Figure 2-3 Delancey Street (Facing North)

2.2.3 DELANCEY STREET / FINUCANE ROAD SIGNALISED INTERSECTION

The Delancey Street / Finucane Road signalised intersection is an existing four-way signalised intersection located at the south-eastern corner of the development site. The existing intersection layout is as demonstrated in the aerial imagery provided in **Figure 2-4**. It should be noted that as a result of the proposal, the Delancey Street / Finucane Road signalised intersection will be upgraded. More details are provided in **Section 4** regarding the proposed upgrades.



Figure 2-4 Existing Delancey Street / Finucane Road Signalised Intersection

3. DETAILS OF THE PROPOSED DEVELOPMENT

This section of the report describes the nature of the proposed development, the proposed access arrangements, servicing arrangements and on-site manoeuvrability.

3.1 PROPOSED DEVELOPMENT

The proposed designation is for an Integrated Health & Education Precinct, consisting of a private hospital, medical consulting suites, research and education facility, residential aged care, and ancillary retail." This report has considered the sites full development potential which also includes childcare, independent living and a community hub which will be assessed at a future date via the DA process. It is understood that the development will be completed over three (3) stages.

The proposed site layout and detailed functional plans, prepared by Destravis Group, are attached in **Appendix A**. We note that these planning layouts have evolved as design of the site has developed and further details of the site gradings and conceptual engineering design has also evolved. As such the current broad land uses have changed as the site constraints have become known.

A summary of the proposed yields as currently proposed are summarised in **Table 3-2**.

USE	YIELDS				
PRIVATE HOSPITAL	148 Beds + 14 Theatres	22,411m ² GFA			
DAY SURGERY	18 Day Beds (inc. within the Private Hospital)				
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	-	4,614 m ² GFA			
CHILD CARE	150 Places	1,200 m² GFA			
SPECIAL RETAIL	-	6,213 m² GFA			
RESEARCH INSTITUTE	-	4,407 m² GFA			
ASSISTED LIVING UNIT & FACILITIES	200 Units + Facilities	25,000 m² GFA			
COMMUNITY HUB	-	2,000 m² GFA			
AGED CARE	80 Beds + Ancillary Area	6,756 m² GFA			
тот	AL	72,601 m² GFA			

Table 3-1 Proposed Development Yields

We note that traffic modelling undertaken for the project has included detailed SIDRA intersection analysis and detailed microsimulation modelling utilising the Aimsun modelling package (see Section 4 of this report). The modelling work undertaken was significant and was based on a previous land use scheme that had estimated a higher development yield that the current proposal. This relates to the areas and number of beds adopted for the Aged Care Facility. The yield adopted in the traffic modelling is shown in **Table 3-2** below and clearly represents a higher development yield and as such a higher level of traffic generation.

On this basis we have not provide new traffic modelling scenarios as the modelling undertaken will inherently include safety margins due to the higher traffic generation rates.

Table 3-2 Proposed Development Yields

USE	YIELD	S			
PRIVATE HOSPITAL	148 Beds + 14 Theatres	22,411m ² GFA			
DAY SURGERY	18 Day Beds				
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	-	4,614 m² GFA			
CHILD CARE	150 Places	1,200 m ² GFA			
SPECIAL RETAIL	-	6,213 m ² GFA			
RESEARCH INSTITUTE	-	4,407 m ² GFA			
ASSISTED LIVING UNIT & FACILITIES	200 Units + Facilities	25,000 m² GFA			
COMMUNITY HUB	-	2,000 m ² GFA			
AGED CARE	134 Beds + Ancillary Area	11,163 m ² GFA			
тот	TOTAL				

3.2 ACCESS ARRANGEMENTS

The proposed development will gain access to the external network via a new signalised intersection on Finucane Road. Finucane Road is gazetted a State Controlled Road and a Limited Access Road along the site frontage.

The development site has an existing priority-controlled, all-movement access provided on Delancey Street, which is designated a local road as per the Redlands City Council. For a development of the nature proposed, it is not considered viable from a traffic engineering perspective to have this access as the sole access into the proposed development. This would lead to safety concerns due to delay and general congestion on Delancey Street and the consequential impacts on the state-controlled road network as drivers becoming impatient. There is additionally limited opportunity to "significantly" upgrade the existing site access on Delancey Street given the constraints associated with land ownership and intersection proximity. It should be noted that as a result of the proposed development, the Delancey Street Access will be upgraded to ban right turns out of the site and to include a 20m left turn lane into the site. A SIDRA intersection analysis of the Delancey Street Access intersection has been completed and has been included in **Section 4**.

The proposed signalised access intersection on Finucane Road will operate efficiently whilst also preserving the functionality, safety and efficiency of the state-controlled road. The intersection design will also ensure vulnerable users such as cyclists and pedestrians are accommodated and protected through the design of this access. A SIDRA intersection analysis of the proposed Finucane Road intersection has been completed and has been included in **Section 4**.

As part of the current conceptual designs, bicycle lanes have been provided on both sides of Finucane Road, and signalised pedestrians crossings have been provided across the site access and the Finucane Road eastern approach. We note that a signalised pedestrian crossing has not been provided on the Finucane Road western approach given there will likely be no pedestrian demand at this location.

The Concept Design for the site access intersection is shown in the plans prepared by Mortons Urban Solutions which have been informed by the detailed traffic modelling work undertaken as part of this project and as described in Section 4 following. The concept design of the intersection is included in **Drawing No. 37801-XWP-005 Amend. A** and included in **Appendix F** of this report.

3.3 LIMITED ACCESS POLICY

As discussed during pre-lodgement meetings with the SARA / DTMR, we note that Finucane Road along the site frontage is a limited access road (LAR 2) and as such the intent of the limited access policy requires consideration and assessment. We note that the current SDAP outlines that access to a road classified as a LAR 2 road can be considered subject to assessment.

Further detailed traffic analysis has been completed and documented further in this traffic report and clearly shows that operationally the proposed access intersection is adequate from an operational perspective for a notional 10 year design horizon.

The new intersection will provide approximately 370m of spacing to the signalised intersection of Finucane Road and Delancey Street which we note is slightly short of the 400m spacing requirement outlined in the Limited Access Policy albeit that we are of the view that the intent is achieved. It is important to note that the location of the proposed signalised intersection has been moved as far west along the site frontage as is physically possible. The location is constrained by the environmental corridor along the western side of the land and the existing bridge structure on Finucane Road.

We note that the proposed turn lanes have been reduced to ensure that the proposed access will not interfere on the ability for the adjacent intersection to be upgraded and that the intersection can be designed to meet the safety and design criteria set out in the Road Planning and Design Manual and/ or Austroads Guides, as per the Limited Access Policy.

4. IMPACT ASSESSMENT

This section provides details of the potential impact that the development traffic could have on the operation of the existing site access and the proposed new site access intersection. In particular, the assessment has focused on the existing Delancev Street priority-controlled access intersection at the north-eastern corner of the site, as well as proposed signalised access intersection on Finucane Road at the southern site boundary.

In addition, we have undertaken a detailed microsimulation modelling exercise, utilising the Aimsun modelling package, for the purposes of assessing the net delays impacts of the proposal on the external road network in accordance with the GTIA principles. This is discussed in more detail in Section 4.7 following.

4.1 **BACKGROUND TRAFFIC**

Vehicle movement survey data was collected at the following intersections on 29th March 2022:

- Finucane Road / McDonald Road; •
- Finucane Road / Dawson Road;
- Finucane Road / Delancey Street; •
- Finucane Road / Wellington Street;
- Wellington Street / Russell Street; •
- Wellington Street / Freeth Street; and •
- Delancey Street / Freeth Street; •

Vehicle movement data for the traffic moving in and out of the existing Delancey Street Access was collected on Thursday 29 October 2020. The through volumes utilised for this intersection were carried from the 2022 counts from the intersections to the north and south.

The intersection count data was collected from 6:00 AM to 9:00 AM and 2:30 PM to 6:30 PM, presented in 15minute periods to capture peak periods.

The AM and PM peak hours for the intersections were found to be as follows:

- AM: 7:45 AM to 8:45 AM; and
- PM: 4:00 PM to 5:00 PM.

Summary of traffic survey data is displayed in Figure C1 attached in Appendix C, with detailed traffic survey data included in Appendix B.

The opening year for the development is proposed to be 2024. To assess the future background traffic a linear growth rate of 1% per annum has been applied to all movements along Finucane Road and Delancey Street. It should be noted that the background traffic entering and exiting the existing site have not been grown. The predicted background traffic in 2024 and 2034 (10-year design horizon) has been presented in Figure C2 and Figure C3 attached in Appendix C.

4.2 IMPACT ASSESSMENT AREA

The impact assessment area that has been included in the network based Aimsun micro-simulation model, which is attached in Appendix D, is as illustrated in Figure 4-1. It should be noted that this micro-simulation traffic model includes the proposed Finucane Road Site Access Intersection.



Figure 4-1 Impact Assessment Area

4.3 TRIP GENERATION

Additional traffic associated with the proposed development has been forecasted, using industry accepted peak hour traffic generation rates, from the RMS (formally RTA) Guide to Traffic Generating Developments and The Department of Transport and Main Roads (DTMR) Open Source Data, for development land uses. The traffic generation rates and in/out directionality of movements adopted for the analysis within this assessment, are shown in **Table 4-1**. For the purposes of this assessment, the speciality retail AM trip generation rate has been taken as 50% of the PM trip generation. The community hub has also been assumed to be ancillary to the development and will not generate individual trips external to the site.

LAND USE	GENERATIO	ON RATE	DIRECTIONALIT	Y (% IN / % OUT)	SOURCE	
LAND USE	AM Peak	PM Peak	AM Peak	PM Peak	SOURCE	
PRIVATE HOSPITAL	-22.07 trips +1.04 trips x	-22.07 trips +1.04 trips	50% in / 50% out	50% in / 50% out	RMS (formally RTA)	
DAY SURGERY	beds	x beds	30 % m / 30 % out	30 % in 7 30 % out		
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	9.81 trips / 100m ² GFA	A 5.80 trips / 100m ² 50% in / 50% out 50% in / 50% o		50% in / 50% out	DTMR Open Source Data	
CHILD CARE	0.8 trips / child	0.7 trips / child	50% in / 50% out	50% in / 50% out	RMS (formally RTA)	
SPECIAL RETAIL	2.8 trips / 100m ² GFA	5.6 trips / 100m ² GFA	50% in / 50% out	50% in / 50% out	RMS (formally RTA)	
RESEARCH INSTITUTE	2 trips / 100m ² GFA	2 trips / 100m ² GFA	90% in / 10% out	10% in / 90% out	RMS (formally RTA)	
ASSISTED LIVING UNIT & FACILITIES	0.2 trips per dwelling	0.2 trips per dwelling	80% in / 20% out	20% in / 80% out	RMS (formally RTA)	
AGED CARE	0.2 trips per dwelling	0.2 trips per dwelling	80% in / 20% out	20% in / 80% out	RMS (formally RTA)	
COMMUNITY HUB	-	-	-	-		

Table 4-1 Generation & Directionality Rates

Due to the nature of the site containing various mixed uses, it is expected that there will be cases of drivers visiting more than one (1) use during the same trip. These trips represent the likely cross-utilisation of the site and described as 'linked trips', they generally result in a reduction of trips to and from the site. A 10% reduction has been assumed for the overall trips generated by the development site which will provide a reasonable forecast of potential traffic demands. As such, based on the above guidelines and assumptions, the estimated traffic generated by the proposed development is documented in **Table 4-2**.

LAND USE	YIELD	AM(IN)	AM (OUT)	PM (IN)	PM (OUT)
PRIVATE HOSPITAL	148 BEDS + 14 THEATRES	75	75	75	75
DAY SURGERY	18 Day Beds	75	75	75	75
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	4,614 m ² GFA	226	226	134	134
CHILD CARE	150 PLACES	60	60	53	53
SPECIAL RETAIL	6,213 m ² GFA	87	87	174	174
RESEARCH INSTITUTE	4,407 m ² GFA	79	9	9	79
ASSISTED LIVING UNIT & FACILITIES	200 UNITS + FACILITIES	32	8	8	32
AGED CARE	134 BEDS + Ancillary Area	21	5	5	21
COMMUNITY HUB	2,000 m ² GFA	-	-	-	-
Total Trips per Peak (10%	Cross-Utilisation)	524	424	412	512

Table 4-2 Development Traffic Generation

As illustrated in **Table 4-2**, the proposed development is anticipated to generate approximately 948 trips during AM peak and 924 trips during the PM peak.

4.4 TRIP DISTRIBUTION

The trip distribution adopted within the model is 77%:23% split between the Finucane Road and Delancey Street accesses. The SIDRA intersection assessment adopted the distribution of traffic through the network that was an output from the Aimsun Micro-simulation Model, which has been included in **Appendix D** noting the availability of various route choices through the network.

4.5 DEVELOPMENT TRAFFIC

Based on the trip distribution outlined in **Section 4.4**, the trip generation for the development is displayed in **Figure C4**, attached in **Appendix C**.

It should be noted that due to the proposed upgrades, traffic movements through the network have altered including alterations to the "background" traffic. Where there are multiple routes available, and the proposal includes upgrades to address existing network constraints, it is routine that there will be some diversion of traffic as a result of improvements made to suit other network movement options. It is this diversion of traffic through the network as a result of the proposed upgrades that has resulted in the negative volumes on turning movements within the network.

4.6 DESIGN TRAFFIC

The design traffic scenarios (background traffic plus the development generated traffic) in 2024 and 2034, are displayed in **Figure C6** and **Figure C7**, respectively attached in **Appendix C**.

4.7 IMPACT ASSESSMENT BASED ON MICROSIM MODELLING

Due to the volume of traffic that the proposed development site is anticipated to generate in the peak hours, the proposed development traffic was assessed in relation to the potential for it to impact over a broader area along the state-controlled and council-controlled road networks. The extent of the potential impacts included a broader "network" of road where various route choices were available which. As such, an assessment of the existing road network, including all stages of the development, was completed to analyse the operational impacts on the surrounding road network based on Aimsun Micro-simulation as opposed to relying solely on SIDRA based analysis. The micro-simulation model has been built with the existing (2022) surrounding road network traffic demand, as outlined in **Section 4.1**. The micro-simulation modelling report prepared by Adanner Pty Ltd has been attached in **Appendix D**.

The micro-simulation model has applied the background traffic (growth rate=1%), trip generation, trip distribution, development traffic and design traffic highlighted above in **Section 4.2**, **Section. 4.3**, **Section 4.4** and **Section 4.5**.

As part of the modelling of the development impacts the Aimsun micro-simulation model has been utilised to test with and without development scenarios along with the testing of various road network improvements that were being contemplated. This allowed the project team to identify the most effective mitigation strategies that would deliver the maximum benefit to the existing road users and would mitigate the impacts of the development and the new signalised intersection proposed on Finucane Road. The vehicle delay assessments were based on the microsimulation modelled outputs and importantly, the methodology associated with the delay assessment appropriately dealt with the redirected "background traffic" applying the "with development" network delays for the "base case" and "upgraded" networks to the background traffic only. This was done by essentially applying different vehicle types to the background and development traffic so they could be easily separated from within the micro-simulation model. This approach is consistent with the GTIA requirements.

Table 4-3 shows an aggregate delay assessment of the network in the existing configuration (no mitigation upgrades), undertaken in accordance with the GTIA. The average vehicle delays for each of the individual intersections within the analysis can be found in the micro-simulation modelling report included in **Appendix D**.

INTERSECTION	PEAK PERIOD	INTERSECT	EGATE 'ION-DELAY -MINUTES)	$ID = \sum_{i=1}^n WD - \sum_{i=1}^n BC$		
		∑ BC	$\sum w d$	Δ	%	
	AM Peak	629,062	805,784	176,722	28.09%	
Existing Configuration (No upgrades)	PM Peak	532,573	618,853	86,280	16.2%	
(Combined Peaks	1,161,635	1,424,637	263,002	22.64%	

Table 1 2 Dela	y Assessment –	Evicting	Configuration
Table 4-5 Dela	y Assessment –	EXISTING	Connguration

The assessment found that with no mitigation upgrades the development generated traffic would increase the aggregate intersection delay by more than 5%. As such, mitigation works to the network are required.

Due to its proximity to the proposed development site, the Finucane Road / Delancey Street signalised intersection was analysed to determine if mitigation upgrades could be undertaken to improve the performance of the overall surrounding network.

We propose that the form of the Finucane Road / Delancey Street intersection be designed generally as shown in **Figure 4-2**. Due to traffic volumes and logistically fitting the intersection into the available road reserve, the following intersection configuration in our view is ideal.

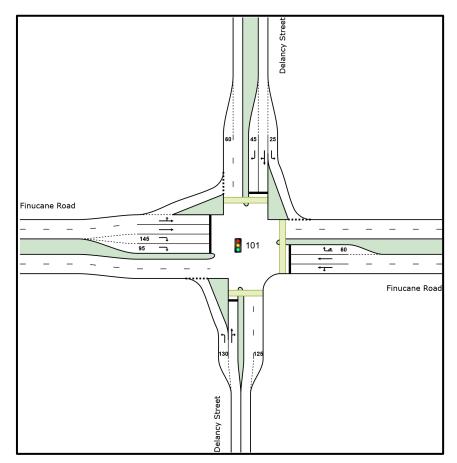


Figure 4-2 Proposed Finucane Road / Delancey Street Signalised Intersection Layout

Table 4-4 shows an aggregate delay assessment of the network including the above mitigation upgrades to the Finucane Road / Delancey Street signalised intersection, undertaken in accordance with the GTIA. It should be noted that the reconfiguration of the intersection has resulted in a change to the current signal phasing plan. Details of the updated signal phasing are included in the micro-simulation model report attached in **Appendix D**. The average vehicle delays for each of the individual intersections within the analysis can be found in the micro-simulation modelling report included in **Appendix D**.

Table 4-4	Delay	Assessment –	With	Upgrades
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INTERSECTION	PEAK PERIOD	INTERSECT	EGATE 'ION-DELAY -MINUTES)	$ID = \sum_{i=1}^{n} WD - \sum_{i=1}^{n} BC$		
		∑ BC	$\sum WD$	Δ	%	
	AM Peak	629,062	615,128	-13,934	-2.22%	
Mitigation (With upgrades)	PM Peak	532,573	433,418	-99,155	-18.62%	
(Combined Peaks	1,161,635	1,048,546	-113,089	-9.74%	

As demonstrated in **Table 4-4**, the assessment found that the development generated traffic would increase the aggregate intersection delay by less than 5% if mitigation upgrades are undertaken to the Finucane Road / Delancey Street signalised intersection, as illustrated in **Figure 4-2**. The results demonstrate that the performance of the network would theoretically improve with the proposed mitigation upgrades, with a decrease in the average delay experienced through the network.

On the basis of the above, and with the implementation of the proposed upgrade works at the intersection of Finucane Road / Delancey Street, in our view that project will result in a "no net worsening" of delay to the

state-controlled road network and on this basis will satisfy the requirements of the SDAP Codes and the GTIA analysis process and guidelines.

The implementation of the proposed upgrade has been contemplated in some detail given the existing corridor widths available and the land ownership constraints. It is noted that the verge on the southern side of the Finucane Road frontage adjacent to the intersection is relatively narrow and while this land appears to be relatively unconstrained it is understood to be owned by the State.

In advancing the conceptual design of the proposed intersection upgrades we note that the project team has developed a solution that appears to be able to be accommodated within the existing road corridor or within land that forms part of the proposal.

The Concept Design for the proposed upgrade works at the subject intersection is shown in the plans prepared by Mortons Urban Solutions which have been informed by the detailed traffic modelling work undertaken and documented above. The concept design of the intersection is included in **Drawing No. 37801-XWP-005 Amend. A** and included in **Appendix F** of this report.

It is proposed that these works will be implemented as part of the ultimate project delivery.

4.8 ACCESS ARRANGEMENTS - IMPACT ASSESSMENT

4.8.1 SIDRA ANALYSIS ASSUMPTIONS

The following scenario provides an assessment of the proposed Finucane Road signalised access intersection as well as the existing priority-controlled access intersection via Delancey Street.

The following parameters have been applied to the SIDRA model:

- A Saturation Flow of 1,950 through car units per hour;
- A Peak Flow Period of 30 minutes, with a conservative Peak Flow Factor of 95%;
- Heavy vehicle volumes based on the traffic survey count data;
- Intersection geometry based on measurements of aerial imagery supplemented with on-site measurements; and
- Gap acceptance parameters based on default SIDRA values (for the right turn out of Arthur Street at the Delancey Street site access intersection, the gap acceptance parameters outlined in AUSTROADS GTRD Part 4A Table 3.5 for a right turn from minor road across a two-lane / two-way road have been adopted).

4.8.2 EXISTING SITE ACCESS ON DELANCEY STREET

SIDRA Intersection software has been used to assess the performance of the Finucane Road signalised access intersection. **Figure 4-3** shows the proposed intersection configuration as modelled in the SIDRA analysis.

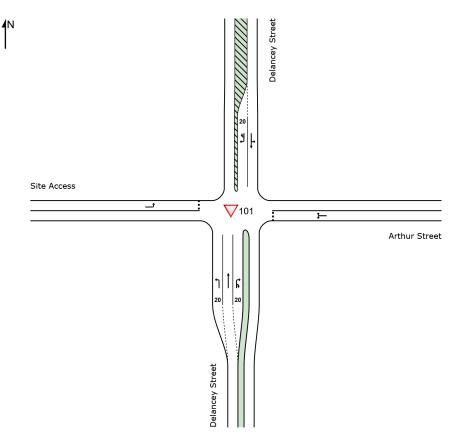


Figure 4-3 Delancey Street / Site Access / Arthur Street Intersection Layout

Table 4-5 presents the results of the Delancey Street access intersection SIDRA analysis for 2024 and 2034 with development scenarios, with detailed movement summary SIDRA outputs included in **Appendix E**.

		AM PEAK				PM PEAK				
SCENARIO	APPF	APPROACH		Avg Delay (s)	LoS	95% Back of Queue (m)	DoS	Avg Delay (s)	LoS	95% Back of Queue (m)
		Left	0.045	5.6	LOS A	0	0.034	5.6	LOS A	0
		Through	0.182	0	LOS A	0	0.174	0	LOS A	0
	Delancey Street (S)	Right	0.008	7.7	LOS A	0.2	0.01	6.9	LOS A	0.3
		U-turn	0.008	11.7	LOS B	0.2	0.01	9.8	LOS A	0.3
		Approach	0.182	1.2	NA	0.2	0.174	1.1	NA	0.3
		Left	0.024	8.6	LOS A	0.5	0.014	7.5	LOS A	0.3
2024 Design	Arthur Street (E)	Right	0.024	18	LOS C	0.5	0.014	14.6	LOS B	0.3
(Background +		Approach	0.024	11.4	LOS B	0.5	0.014	9.3	LOS A	0.3
Development)		Left	0.301	5.6	LOS A	0	0.225	5.6	LOS A	0
		Through	0.301	0.1	LOS A	0	0.225	0.1	LOS A	0
	Delancey Street (N)	Right	0.111	7.6	LOS A	3.1	0.078	7.3	LOS A	2.1
		U-turn	0.111	9.3	LOS A	3.1	0.078	9.1	LOS A	2.1
		Approach	0.301	1.2	NA	3.1	0.225	1.2	NA	2.1
	Site Access (W)	Left	0.143	7.3	LOS A	3.7	0.199	7.4	LOS A	5.5
		Approach	0.143	7.3	LOS A	3.7	0.034	5.6	LOS A	0
		Left	0.045	5.6	LOS A	0	0.034	5.6	LOS A	0
		Through	0.244	0.1	LOS A	0	0.194	0	LOS A	0
	Delancey Street (S)	Right	0.011	8.2	LOS A	0.3	0.011	7.1	LOS A	0.3
		U-turn	0.011	12.7	LOS B	0.3	0.011	10.2	LOS B	0.3
		Approach	0.244	1	NA	0.3	0.194	1	NA	0.3
		Left	0.037	9.2	LOS A	0.8	0.017	7.7	LOS A	0.4
2034 Design	Arthur Street (E)	Right	0.037	23.6	LOS C	0.8	0.017	16.2	LOS C	0.4
(Background		Approach	0.037	14	LOS B	0.8	0.017	9.6	LOS A	0.4
- Development)		Left	0.335	5.6	LOS A	0	0.245	5.6	LOS A	0
		Through	0.335	0.1	LOS A	0	0.245	0.1	LOS A	0
	Delancey Street (N)	Right	0.129	8.5	LOS A	3.5	0.082	7.6	LOS A	2.2
		U-turn	0.129	10.7	LOS B	3.5	0.082	9.5	LOS A	2.2
		Approach	0.335	1.3	NA	3.5	0.245	1.1	NA	2.2
		Left	0.166	8.2	LOS A	4.2	0.208	7.6	LOS A	5.7
	Site Access (W)	Approach	0.045	5.6	LOS A	0	0.034	5.6	LOS A	0

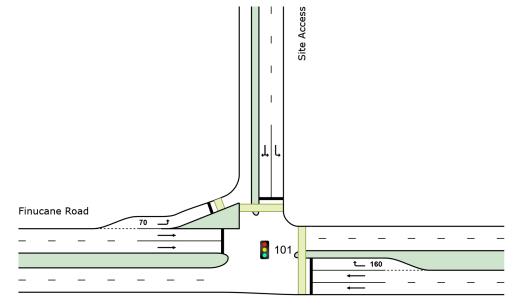
Table 4-5 Delancey Street / Site Access / Arthur Street Intersection SIDRA Outputs

As illustrated in **Table 4-5**, the Delancey Street access will operate below the practical capacity for a prioritycontrolled intersection (DoS<0.8, Delay<42 sec) at both the year of opening (2024) and a 10-year design horizon (2034) in both peak periods.

4.8.3 PROPOSED SITE ACCESS ON FINUCANE ROAD

4Ν

SIDRA Intersection software has been used to assess the performance of the Finucane Road signalised access intersection. **Figure 4-4** shows the proposed intersection configuration as modelled in the SIDRA analysis.



Finucane Road

Figure 4-4 Proposed Finucane Road Signalised Access Intersection

The signal phasing plan adopted in both peak periods can be seen in **Figure 4-5.** A cycle time of 90 seconds has been adopted for this analysis.

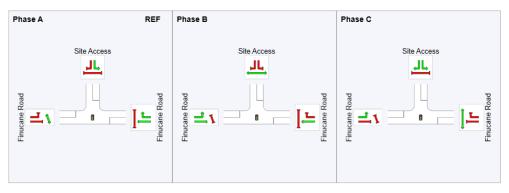


Figure 4-5 Finucane Road Signalised Access Intersection Signal Phasing

Table 4-6 presents the results of the Finucane Road access intersection SIDRA analysis with detailed movement and phasing SIDRA Outputs attached in **Appendix E**.

	APPROACH			AM F	PEAK		PM PEAK			
SCENARIO			DoS	Avg Delay (s)	LoS	95% Back of Queue (m)	DoS	Avg Delay (s)	LoS	95% Back of Queue (m)
		Through	0.595	6.9	LOS A	126	0.601	9.4	LOS A	133.3
	Finucane Road (E)	Right	0.604	47.1	LOS D	45.4	0.633	49.4	LOS D	40.9
		Approach	0.604	10.3	LOS B	126	0.633	12.6	LOS B	133.3
2024 Design		Left	0.212	27.3	LOS C	30	0.249	25.5	LOS C	37.4
(Background +	Site Access (N)	Right	0.517	44.5	LOS D	43.7	0.612	41.1	LOS D	68.6
Development)		Approach	0.517	36.2	LOS D	43.7	0.612	34.4	LOS C	68.6
	Finucane Road (W)	Left	0.178	9.5	LOS A	22.4	0.143	8.8	LOS A	16.5
		Through	0.620	16.7	LOS B	136.5	0.623	18.7	LOS B	132.6
		Approach	0.62	15.5	LOS B	136.5	0.623	17.2	LOS B	132.6
		Through	0.637	7.3	LOS A	142.1	0.641	8.8	LOS A	147.3
	Finucane Road (E)	Right	0.659	48.9	LOS D	46.7	0.633	49.4	LOS D	40.9
		Approach	0.659	10.6	LOS B	142.1	0.641	11.7	LOS B	147.3
2034 Design		Left	0.219	28.1	LOS C	30.5	0.264	27.1	LOS C	38.8
(Background +	Site Access (N)	Right	0.670	46.4	LOS D	59.2	0.684	44.1	LOS D	72.1
Development)		Approach	0.67	38.7	LOS D	59.2	0.684	36.8	LOS D	72.1
		Left	0.175	9.2	LOS A	21.4	0.143	8.8	LOS A	16.5
	Finucane Road (W)	Through	0.713	17.1	LOS B	170.8	0.695	18.2	LOS B	159.6
		Approach	0.713	15.9	LOS B	170.8	0.695	17	LOS B	159.6

Table 4-6 Finucane Road Signalised Access Intersection SIDRA Outputs

As illustrated in **Table 4-6**, the Finucane Road access will operate within the reasonable operational limits for a signalised intersection (notionally DoS<0.9) at both the year of opening (2024) and a 10-year design horizon (2034) in both peak periods. Queues on the eastern Finucane Road approach will remain within turn lanes and clear of the adjacent Delancey Street signalised intersection. Queues on the western Finucane Road approach will remain within turn lanes and clear of adjacent intersections.

The operational analysis shows that the proposed signalised intersection would operate at an acceptable level for the ten-year design horizon and importantly all traffic would be able to clear during each relevant phase.

We note that this analysis has been based on the intersection being analysed as an isolated intersection and not part of a network. The micro-simulation model report includes the proposed Finucane Road access intersection, demonstrating that a network-based analysis results in even better operational outcomes that that demonstrated in **Table 4-6**.

The Concept Design for the site access intersection is shown in the plans prepared by Mortons Urban Solutions which have been informed by the detailed traffic modelling work undertaken and described above. The concept design of the intersection is included in **Drawing No. 37801-XWP-005 Amend. A** and included in **Appendix F** of this report.

5. SUMMARY

Lambert & Rehbein (SEQ) Pty Ltd has been commissioned by The Hub Precinct Pty Ltd undertake a Traffic Impact Assessment for a proposed mixed use commercial development located at 58-68 Delancey Street, Ormiston QLD 4160, which is formally described as 0/SP308738, 0-2/SP308739; 0/SP308740; 4/SP308740; and 10-16/SP314782. The development site currently contains existing and approved medical and education buildings, a car park, and a portion of vacant land, with a total site area of approximately 5.2ha.

The proposed designation is for an Integrated Health & Education Precinct, consisting of a private hospital, medical consulting suites, research and education facility, residential aged care, and ancillary retail." This report has considered the sites full development potential which also includes childcare, independent living and a community hub which will be assessed at a future date via the DA process. It is understood that the development will be completed over three (3) stages. The proposed development proposes to gain access to the external road network via a new signalised intersection on Finucane Road and an existing access on Delancey Street. The proposed site layout, prepared by Destravis Group, is attached in **Appendix A**.

The development application is being pursued through the Ministerial Designation process and this traffic assessment forms part of the application material.

The development proposes to gain access directly to the State-controlled road network via a new signalised intersection on Finucane Road to the west of the existing Delancey Street intersection. The site will also gain access to the Delancey Street albeit that the existing all-movements access will be reconfigured to remove the right turn egress from the subject land.

Given the intent to provide a new signalised intersection on Delancey Street, the traffic assessment has included undertaking a detailed assessment of the site intersections for a 10 year design horizon beyond the potential year of opening of 2024. This analysis indicated that the proposal for the new signalised intersection can be incorporated into the state-controlled road network in a manner that will achieve an acceptable level of operational efficiency and can be delivered in a manner consistent with the applicable guidelines. Conceptual functional layout plans have been developed for the site access intersection and are shown in **Drawing No. 37801-XWP-005 Amend. A** and included in **Appendix F** of this report.

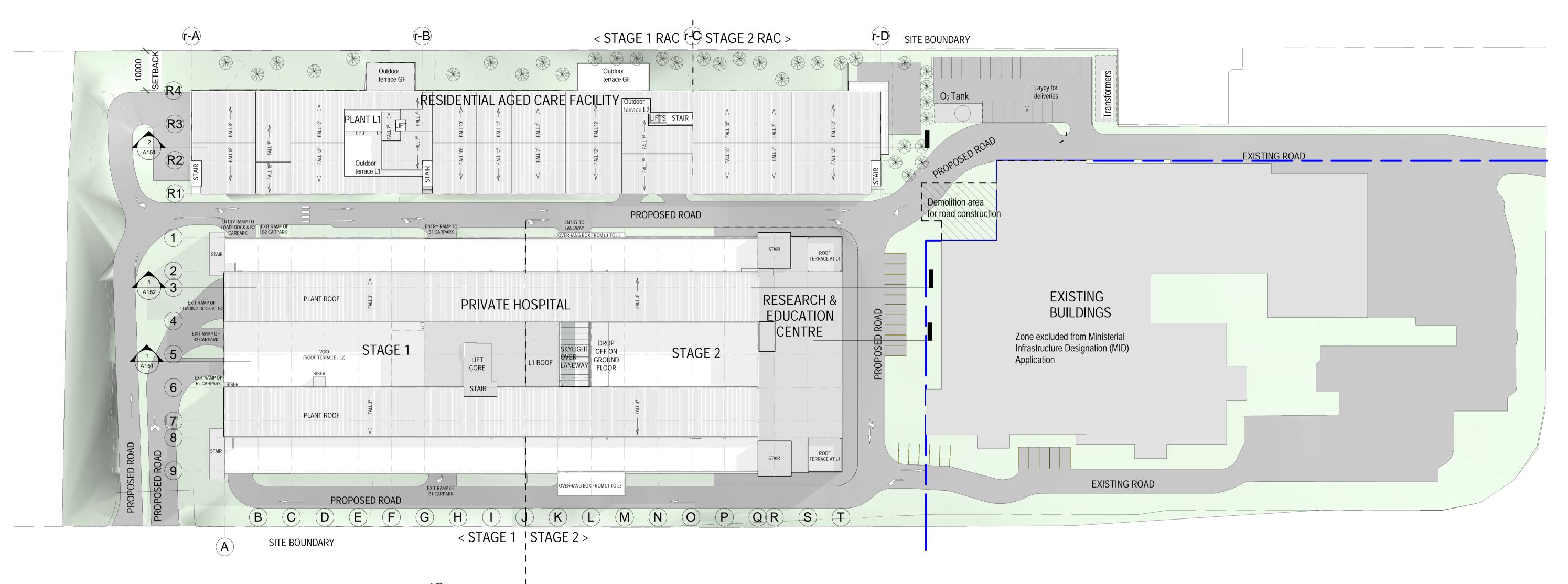
On the basis of the development including a new intersection on the state-controlled road network we have also undertaken a detailed traffic modelling assessment based on a detailed microsimulation modelling approach. This analysis determined that the development, along with the new signalised intersection would have a relatively significant impact on the net delays through the network. As a part of the modelling process assessment of suitable road network options were undertaken and an upgraded layout for the Delancey Street signalised intersection was developed to mitigate the development impacts. The changes proposed can be delivered within the road network, within any land take contained with land that forms part of the application, and deliver a significant net benefit to the network-based delays.

Conceptual functional layout plans for the upgrades at the Finucane Road / Delancey Street intersection and are shown in **Drawing No. 37801-XWP-005 Amend. A** and included in **Appendix F** of this report.

Based on the detailed traffic analysis and modelling undertaken we are of the view that the proposal can be supported from a traffic perspective.



APPENDIX A – SITE PLANS



FINUCANE ROAD

DESTRAVIS GROUP

Plot Date: 27/04/2023 10:24:37 AM

This drawing and design is subject to copyright [©] and may not be reproduced without prior consent.

Contractor to verify all dimensions on site before commencing work.

Report all discrepancies to the

preference to scaled drawings.

File Path: D:/Destravis Dropbox/DES_Projects/Hub 68_Cleveland MIDIA15_Working Drawings/230427 - HUB68 Centre of Excellence - Aging & Welling

principal consultant prior to construction

Figured dimensions to be taken in

PRELIMINARY

CLARIFICATIONS AND DISCLAIMERS

THE MAXIMUM BUILDING HEIGHT VARIES IN RELATION TO THE ADJACENT AT GRADE GROUND LEVEL DUE TO SITE TOPOGRAPHY
 REFER TO BUILDING SECTIONS FOR BUILDING HEIGHT INFORMATION, WHICH ARE TO BE READ IN CONJUNCTION WITH SITE PLANS FOR AT GRADE GROUND LEVELS
 ALL PLANS ARE PRELIMINARY AND SUBJECT TO FURTHER DEVELOPMENT OF THE FUNCTIONAL BRIEF AND FINAL DESIGN
 TENANCY SPACES ARE PROVIDED AS "COLD SHELL" AREAS FOR FIT OUT BY TENANTS
 PARKING NUMBERS ARE SUBJECT TO DEVELOPMENT OF THE FINAL DESIGN FINAL DESIGN

 REV
 DESCRIPTION

 1
 Revision 2

 2
 Revision 3

 3
 Coordination and Comments

 4
 Client's comments incorporated

 5
 MID Issue

DATE	
28/02/2023	
10/03/2023	
27/03/2023	
04/04/2023	
26/04/2023	

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CLIENT THE HUB PRECINCT PTY LTD PROJECT NAME status MID ISSUE DRAWING STG2 - WHOLE S

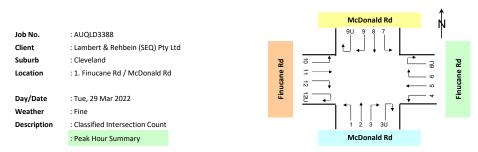
LOCATION 58-68 DELANCEY STREET, ORMISTON QLD 4160

Hub68 Centre of Excellence - Aging & Wellness

0 5 10 15 m

SCALE 1.000 AT ORIGINAL SIZE		
scale @ A1 1 : 500	JOB No 4_2301_03	NORTH
DATE 26/04/2023	DWG No A140	
DRAWN BY JPS, NB & NS	rev 5	
	1 : 500 DATE 26/04/2023 DRAWN BY	SCALE @ A1 JOB No 1 : 500 4_2301_03 DATE DWG No 26/04/2023 A140 DRAWN BY REV

APPENDIX B- RAW TRAFFIC SURVEY DATA





	Ар	proa	ich	М	Donald	Rd	Fi	nucane l	Rd	М	Donald	Rd	Fi	nucane	Rd	Total
	Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand 1
AM	7:45	to	8:45	140	2	142	1,396	40	1,436	291	6	297	1,171	52	1,223	3,098
PM	16:00	to	17:00	125	2	127	1,269	24	1,293	163	0	163	1,377	18	1,395	2,978

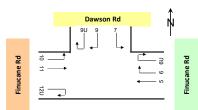
otal	Rd	nucane	Fi	Rd	Donald	М	Rd	nucane l	Fi	Rd	:Donald	Mo	ch	proa	Ap
Grand Total	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	iod	e Per	Tim
1,73	586	32	554	93	6	87	1,000	21	979	51	1	50	7:00	to	6:00
1,89	659	35	624	95	5	90	1,079	23	1,056	59	0	59	7:15	to	6:15
2,12	764	41	723	114	4	110	1,185	26	1,159	60	0	60	7:30	to	6:30
2,23	847	39	808	123	3	120	1,204	31	1,173	61	0	61	7:45	to	6:45
2 2,46	1,002	49	953	156	3	153	1,237	31	1,206	69	0	69	8:00	to	7:00
1 2,74	1,151	50	1,101	211	6	205	1,294	36	1,258	85	0	85	8:15	to	7:15
2 2,96	1,212	50	1,162	261	7	254	1,373	38	1,335	115	1	114	8:30	to	7:30
3 3,09	1,223	52	1,171	297	6	291	1,436	40	1,396	142	2	140	8:45	to	7:45
2 3,09	1,162	44	1,118	286	6	280	1,499	48	1,451	144	2	142	9:00	to	8:00
0 7,28	2,750	125	2,625	535	15	520	3,736	100	3,636	264	3	261	als	1 Tota	AN
5 2,73	1,265	36	1,229	175	3	172	1,164	28	1,136	132	3	129	15:30	to	14:30
4 2,83	1,314	30	1,284	182	5	177	1,206	31	1,175	137	2	135	15:45	to	14:45
1 2,88	1,321	39	1,282	193	8	185	1,252	35	1,217	120	1	119	16:00	to	15:00
5 2,87	1,335	35	1,300	190	7	183	1,222	34	1,188	131	1	130	16:15	to	15:15
4 2,90	1,334	33	1,301	187	6	181	1,263	32	1,231	124	2	122	16:30	to	15:30
0 2,89	1,330	30	1,300	171	3	168	1,275	34	1,241	118	2	116	16:45	to	15:45
5 2,97	1,395	18	1,377	163	0	163	1,293	24	1,269	127	2	125	17:00	to	16:00
8 2,96	1,378	15	1,363	153	0	153	1,312	21	1,291	122	2	120	17:15	to	16:15
6 2,90	1,386	14	1,372	156	0	156	1,242	20	1,222	118	1	117	17:30	to	16:30
9 2,81	1,349	14	1,335	171	0	171	1,174	15	1,159	120	0	120	17:45	to	16:45
2 2,59	1,292	14	1,278	156	0	156	1,037	16	1,021	114	0	114	18:00	to	17:00
7 2,40	1,207	13	1,194	149	0	149	939	13	926	114	0	114	18:15	to	17:15
6 2,11	1,066	8	1,058	141	0	141	810	17	793	101	0	101	18:30	to	17:30
1 10,66	5,051	91	4,960	659	9	650	4,479	97	4,382	475	6	469	als	1 Tota	PN

 Job No.
 : AUQLD3388

 Client
 : Lambert & Rehbein (SEQ) Pty Ltd

 Suburb
 : Cleveland

 Location
 : 2. Finucane Rd / Dawson Rd



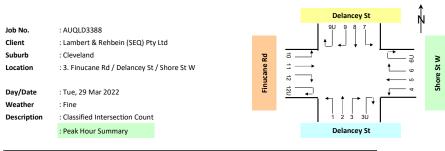


Day/Date: Tue, 29 Mar 2022Weather: FineDescription: Classified Intersect

: Classified Intersection Count : Peak Hour Summary

	Approach	Fi	nucane l	۲d	D	awson F	۲d	Fi	nucane	Rd	Total
	Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand 1
AM	7:45 to 8:45	1,501	41	1,542	97	4	101	1,431	51	1,482	3,125
PM	16:00 to 17:00	1,372	30	1,402	53	1	54	1,450	12	1,462	2,918

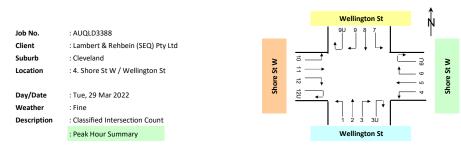
Appr	oach
Time P	eriod
6:00 to	o 7:00
6:15 to	7:15
6:30 to	7:30
6:45 to	
7:00 to	
7:15 to	
7:30 to	
7:45 to	
8:00 to	
AM T	otals
14:30 to	15:3
14:45 to	15:4
15:00 to	16:0
15:15 to	16:1
15:30 to	16:3
15:45 to	
.6:00 to	
6:15 to	
.6:30 to	
5:45 to	
:00 to	
5 to	
0 to	18:3
PM Te	otals





	Ар	proa	ch	D	elancey	St	s	hore St \	N	D	elancey	St	Fi	nucane I	۲d	Total
	Tim	e Pei	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
AM	7:45	to	8:45	727	10	737	890	32	922	511	11	522	1,492	53	1,545	3,726
РМ	16:00	to	17:00	652	10	662	859	18	877	417	3	420	1,443	13	1,456	3,415

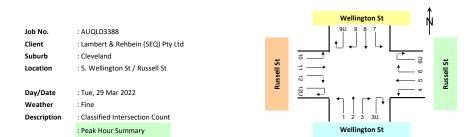
otal	Rd	nucane	Fi	St	elancey	D	N	hore St \	s	St	elancey	De	ch	proa	Ap
Grand Total	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	iod	e Per	Tim
1,837	673	30	643	156	4	152	566	12	554	442	8	434	7:00	to	6:00
2,128	762	43	719	196	5	191	682	16	666	488	9	479	7:15	to	6:15
2,328	855	35	820	245	5	240	719	24	695	509	11	498	7:30	to	6:30
2,537	972	40	932	265	5	260	729	22	707	571	15	556	7:45	to	6:45
2,871	1,149	52	1,097	344	5	339	759	26	733	619	13	606	8:00	to	7:00
3,231	1,352	43	1,309	418	8	410	773	27	746	688	15	673	8:15	to	7:15
3,556	1,510	56	1,454	493	10	483	830	25	805	723	12	711	8:30	to	7:30
3,726	1,545	53	1,492	522	11	511	922	32	890	737	10	727	8:45	to	7:45
3,609	1,469	47	1,422	505	13	492	915	36	879	720	14	706	9:00	to	8:00
8,317	3,291	129	3,162	1,005	22	983	2,240	74	2,166	1,781	35	1,746	als	1 Tota	AN
3,303	1,476	39	1,437	372	6	366	864	27	837	591	8	583	15:30	to	14:30
3,377	1,466	40	1,426	457	10	447	835	30	805	619	13	606	15:45	to	14:45
3,402	1,485	41	1,444	488	12	476	814	28	786	615	11	604	16:00	to	15:00
3,400	1,457	37	1,420	451	12	439	812	27	785	680	10	670	16:15	to	15:15
3,341	1,429	33	1,396	440	12	428	818	24	794	654	12	642	16:30	to	15:30
3,391	1,464	22	1,442	390	6	384	848	18	830	689	11	678	16:45	to	15:45
3,415	1,456	13	1,443	420	3	417	877	18	859	662	10	652	17:00	to	16:00
3,371	1,482	12	1,470	438	1	437	863	14	849	588	9	579	17:15	to	16:15
3,339	1,460	12	1,448	432	3	429	870	13	857	577	7	570	17:30	to	16:30
3,095	1,415	16	1,399	393	4	389	794	14	780	493	3	490	17:45	to	16:45
2,889	1,370	16	1,354	340	5	335	725	13	712	454	1	453	18:00	to	17:00
2,621	1,253	14	1,239	302	6	296	656	15	641	410	1	409	18:15	to	17:15
2,349	1,151	11	1,140	260	3	257	580	12	568	358	3	355	18:30	to	17:30
12,332	5,516	95	5,421	1,504	24	1,480	3,132	76	3,056	2,180	30	2,150	als	1 Tota	PN





	Ар	proa	ch	W	ellington	St	S	hore St \	N	w	ellingtor	St	S	hore St \	N	Total
	Tim	e Pei	riod	Lights	Heavies	Total	Grand T									
АМ	8:00	to	9:00	589	33	622	938	32	970	823	19	842	1,119	30	1,149	3,583
РМ	16:15	to	17:15	619	8	627	1,011	21	1,032	603	9	612	1,068	9	1,077	3,348

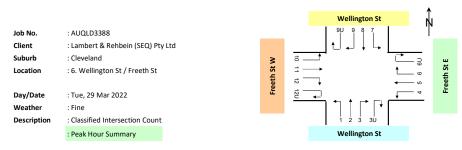
otal	w	hore St	s	St	ellingtor	w	N	hore St \	s	St	ellington	We	ch	proa	Ар
Grand Total	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	iod	e Per	Tim
1,659	449	26	423	370	4	366	529	15	514	311	28	283	7:00	to	6:00
1,849	500	33	467	424	4	420	581	14	567	344	19	325	7:15	to	6:15
2,105	586	36	550	498	3	495	647	19	628	374	25	349	7:30	to	6:30
2,359	674	33	641	570	9	561	694	20	674	421	26	395	7:45	to	6:45
2,618	804	41	763	627	13	614	737	20	717	450	26	424	8:00	to	7:00
2,924	937	41	896	713	16	697	789	26	763	485	27	458	8:15	to	7:15
3,211	1,060	34	1,026	758	20	738	859	23	836	534	27	507	8:30	to	7:30
3,494	1,156	38	1,118	830	19	811	917	25	892	591	30	561	8:45	to	7:45
3,583	1,149	30	1,119	842	19	823	970	32	938	622	33	589	9:00	to	8:00
7,860	2,402	97	2,305	1,839	36	1,803	2,236	67	2,169	1,383	87	1,296	als	l Tota	AN
3,142	1,006	31	975	627	17	610	938	24	914	571	21	550	15:30	to	14:30
3,256	1,115	25	1,090	648	17	631	916	21	895	577	16	561	15:45	to	14:45
3,278	1,135	30	1,105	690	19	671	883	19	864	570	15	555	16:00	to	15:00
3,292	1,131	25	1,106	718	20	698	845	16	829	598	12	586	16:15	to	15:15
3,316	1,138	26	1,112	691	17	674	870	14	856	617	12	605	16:30	to	15:30
3,307	1,036	23	1,013	669	14	655	975	20	955	627	13	614	16:45	to	15:45
3,316	1,030	12	1,018	641	12	629	999	20	979	646	10	636	17:00	to	16:00
3,348	1,077	9	1,068	612	9	603	1,032	21	1,011	627	8	619	17:15	to	16:15
3,331	1,096	9	1,087	626	8	618	1,005	21	984	604	6	598	17:30	to	16:30
3,246	1,088	10	1,078	615	5	610	938	20	918	605	4	601	17:45	to	16:45
3,096	1,067	12	1,055	612	4	608	856	16	840	561	7	554	18:00	to	17:00
2,900	985	12	973	621	4	617	771	16	755	523	8	515	18:15	to	17:15
2,646	914	9	905	554	3	551	694	14	680	484	5	479	18:30	to	17:30
12,435	4,154	75	4,079	2,498	45	2,453	3,507	73	3,434	2,276	44	2,232	als	l Tota	PN





	Ар	proa	ich	W	ellington	St	F	Russell S	t	w	ellingtor	St	F	Russell S	t	Total
	Tim	e Pei	riod	Lights	Heavies	Total	Grand 1									
АМ	7:45	to	8:45	713	23	736	497	9	506	622	27	649	447	10	457	2,348
РМ	16:00	to	17:00	742	9	751	361	7	368	566	12	578	519	4	523	2,220

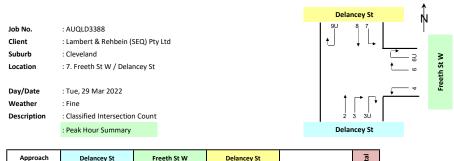
otal	it	Russell S		n St	ellingtor	W	t	Russell S	F	St	ellington	We	ch	proa	Ap
Grand Total	Total	Heavies	Lights	iod	e Per	Tim									
1,227	152	4	148	397	17	380	211	3	208	467	25	442	7:00	to	6:00
1,327	195	4	191	389	15	374	244	5	239	499	17	482	7:15	to	6:15
1,484	231	4	227	454	9	445	251	5	246	548	18	530	7:30	to	6:30
1,599	255	3	252	507	7	500	255	3	252	582	26	556	7:45	to	6:45
1,774	276	2	274	575	12	563	313	5	308	610	24	586	8:00	to	7:00
1,996	323	3	320	648	18	630	362	4	358	663	22	641	8:15	to	7:15
2,164	392	6	386	652	21	631	429	7	422	691	26	665	8:30	to	7:30
2,348	457	10	447	649	27	622	506	9	497	736	23	713	8:45	to	7:45
2,296	451	11	440	606	28	578	500	9	491	739	24	715	9:00	to	8:00
5,297	879	17	862	1,578	57	1,521	1,024	17	1,007	1,816	73	1,743	als	1 Tota	AN
1,971	410	11	399	547	28	519	339	4	335	675	22	653	15:30	to	14:30
2,044	413	10	403	560	26	534	361	6	355	710	18	692	15:45	to	14:45
2,057	425	11	414	583	24	559	349	5	344	700	17	683	16:00	to	15:00
2,156	449	12	437	601	20	581	339	4	335	767	14	753	16:15	to	15:15
2,199	472	8	464	605	18	587	351	6	345	771	14	757	16:30	to	15:30
2,199	486	7	479	607	13	594	346	6	340	760	14	746	16:45	to	15:45
2,220	523	4	519	578	12	566	368	7	361	751	9	742	17:00	to	16:00
2,179	544	2	542	600	8	592	336	7	329	699	7	692	17:15	to	16:15
2,120	542	2	540	597	8	589	301	5	296	680	5	675	17:30	to	16:30
1,998	513	2	511	573	8	565	282	2	280	630	4	626	17:45	to	16:45
1,870	480	2	478	558	8	550	233	1	232	599	6	593	18:00	to	17:00
1,762	441	2	439	522	7	515	210	1	209	589	6	583	18:15	to	17:15
1,614	389	3	386	496	5	491	193	1	192	536	6	530	18:30	to	17:30
7,904	1,813	24	1,789	2,245	59	2,186	1,184	16	1,168	2,662	47	2,615	als	l Tota	PN





	Approach		proach Wellington		St	Freeth St E		Wellington St			Freeth St W			Total		
	Tim	e Pei	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
АМ	7:45	to	8:45	563	17	580	4	0	4	684	11	695	212	7	219	1,498
РМ	15:30	to	16:30	728	7	735	39	0	39	524	15	539	162	2	164	1,477

otal	Freeth St W			St	ellington	W	E	reeth St	F	St	ellington	We	Approach		Ap
Grand Total	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	iod	e Per	Tim
731	46	1	45	360	2	358	0	0	0	325	15	310	7:00	to	6:00
849	63	1	62	419	1	418	3	0	3	364	18	346	7:15	to	6:15
966	73	1	72	476	3	473	6	0	6	411	19	392	7:30	to	6:30
1,079	74	1	73	537	7	530	6	0	6	462	23	439	7:45	to	6:45
1,193	104	2	102	601	12	589	6	0	6	482	20	462	8:00	to	7:00
1,291	124	1	123	637	13	624	5	0	5	525	17	508	8:15	to	7:15
1,436	173	6	167	692	12	680	2	0	2	569	16	553	8:30	to	7:30
1,498	219	7	212	695	11	684	4	0	4	580	17	563	8:45	to	7:45
1,449	221	7	214	649	12	637	4	0	4	575	18	557	9:00	to	8:00
3,373	371	10	361	1,610	26	1,584	10	0	10	1,382	53	1,329	als	1 Tota	AN
1,373	139	3	136	566	14	552	9	1	8	659	11	648	15:30	to	14:30
1,406	168	3	165	564	17	547	10	1	9	664	9	655	15:45	to	14:45
1,455	166	2	164	594	19	575	19	0	19	676	9	667	16:00	to	15:00
1,469	161	2	159	553	15	538	34	0	34	721	7	714	16:15	to	15:15
1,477	164	2	162	539	15	524	39	0	39	735	7	728	16:30	to	15:30
1,471	142	2	140	534	12	522	39	0	39	756	7	749	16:45	to	15:45
1,421	127	2	125	513	9	504	32	0	32	749	6	743	17:00	to	16:00
1,465	144	2	142	532	8	524	14	0	14	775	6	769	17:15	to	16:15
1,447	130	1	129	540	3	537	10	0	10	767	6	761	17:30	to	16:30
1,400	113	1	112	505	2	503	9	0	9	773	5	768	17:45	to	16:45
1,396	118	1	117	517	2	515	15	0	15	746	7	739	18:00	to	17:00
1,254	87	1	86	470	0	470	25	0	25	672	7	665	18:15	to	17:15
1,165	81	1	80	423	0	423	25	0	25	636	7	629	18:30	to	17:30
5,462	514	7	507	2,068	32	2,036	83	1	82	2,797	31	2,766	als	1 Tota	PN





	Approach			Delancey St			Freeth St W			Delancey St		
	Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
AM	7:45	to	8:45	438	17	455	182	3	185	596	12	608
РМ	14:45	to	15:45	440	11	451	167	6	173	386	8	394

otal	St	Delancey St			eeth St	Fr	St	elancey	D	ch	proa	Ap
Grand Total	Total	Heavies	Lights	Total	Heavies	Lights	Total	Heavies	Lights	riod	e Per	Tim
324	162	3	159	61	3	58	101	0	101	7:00	to	6:00
426	207	2	205	83	3	80	136	2	134	7:15	to	6:15
496	252	2	250	92	2	90	152	3	149	7:30	to	6:30
578	285	3	282	105	2	103	188	4	184	7:45	to	6:45
714	347	3	344	128	2	126	239	8	231	8:00	to	7:00
874	405	4	401	142	3	139	327	9	318	8:15	to	7:15
1,088	507	11	496	163	3	160	418	14	404	8:30	to	7:30
1,248	608	12	596	185	3	182	455	17	438	8:45	to	7:45
1,201	583	12	571	177	4	173	441	15	426	9:00	to	8:00
2,239	1,092	18	1,074	366	9	357	781	23	758	als	1 Tota	AN
886	296	4	292	169	5	164	421	9	412	15:30	to	14:30
1,018	394	8	386	173	6	167	451	11	440	15:45	to	14:45
1,004	449	8	441	158	8	150	397	10	387	16:00	to	15:00
934	406	10	396	149	6	143	379	10	369	16:15	to	15:15
899	399	11	388	135	3	132	365	9	356	16:30	to	15:30
871	350	5	345	152	2	150	369	6	363	16:45	to	15:45
891	356	4	352	153	0	153	382	4	378	17:00	to	16:00
925	390	1	389	155	0	155	380	3	377	17:15	to	16:15
937	378	3	375	176	2	174	383	4	379	17:30	to	16:30
843	320	4	316	172	2	170	351	5	346	17:45	to	16:45
782	277	4	273	179	3	176	326	6	320	18:00	to	17:00
704	244	5	239	168	3	165	292	6	286	18:15	to	17:15
591	204	2	202	140	1	139	247	4	243	18:30	to	17:30
3,313	1,277	20	1,257	620	11	609	1,416	26	1,390	als	1 Tota	PN

APPENDIX C- TRAFFIC MOVEMENT DIAGRAMS

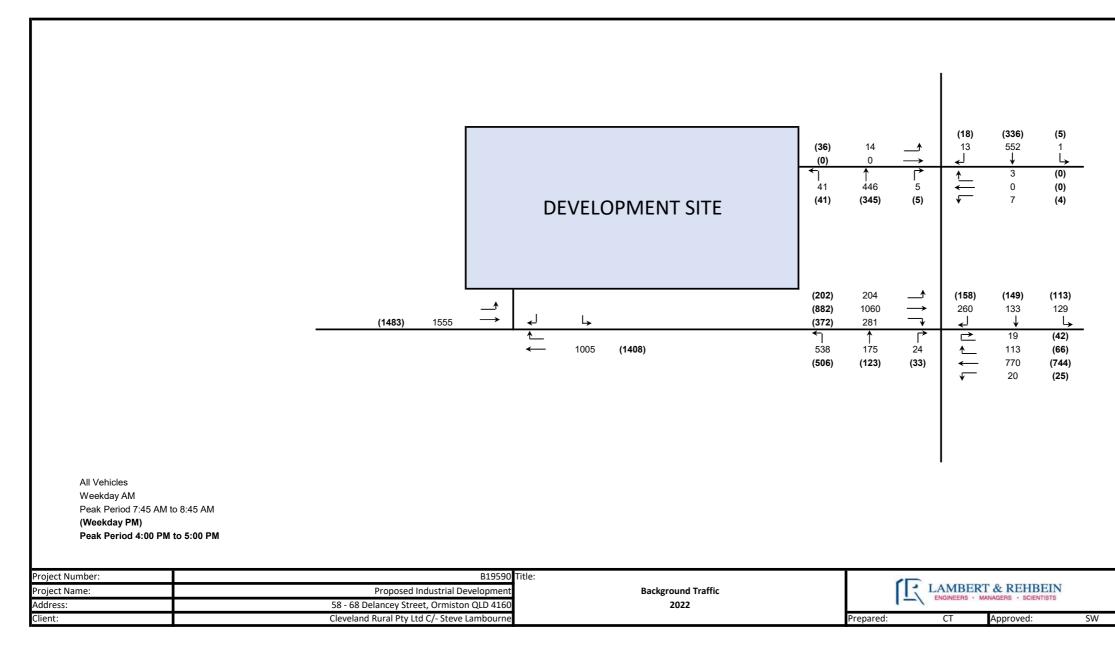
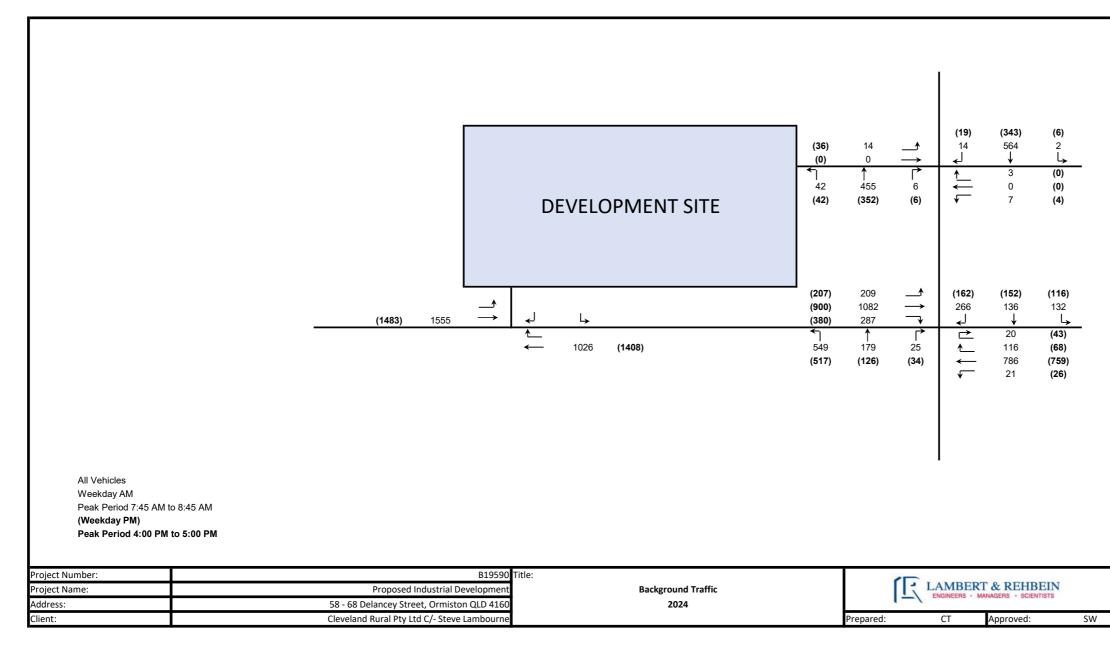
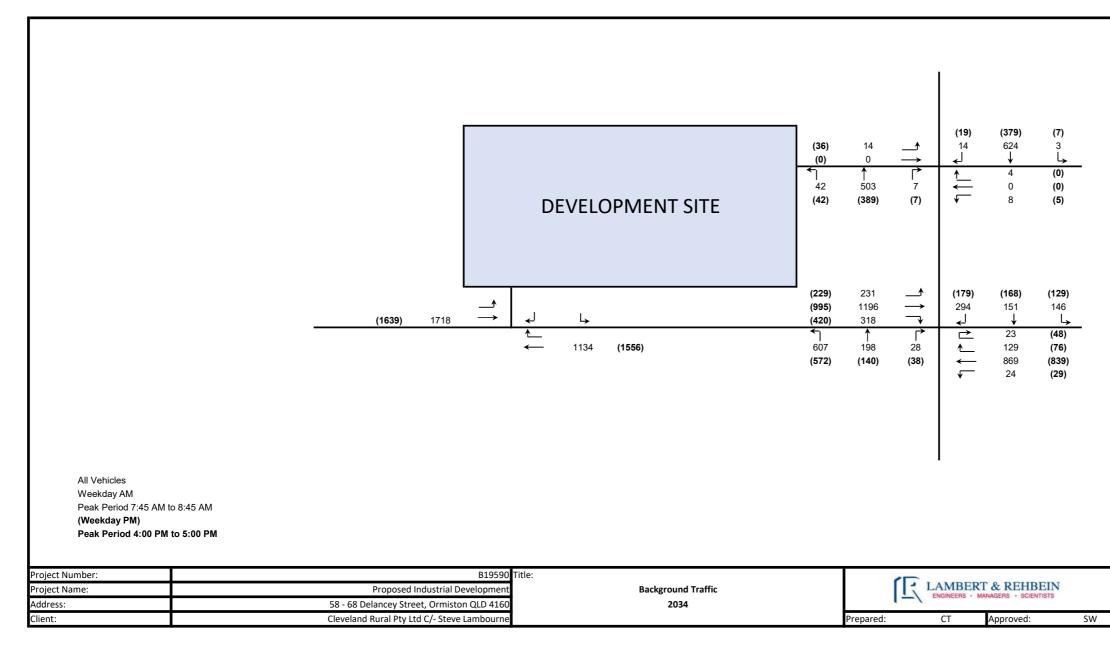
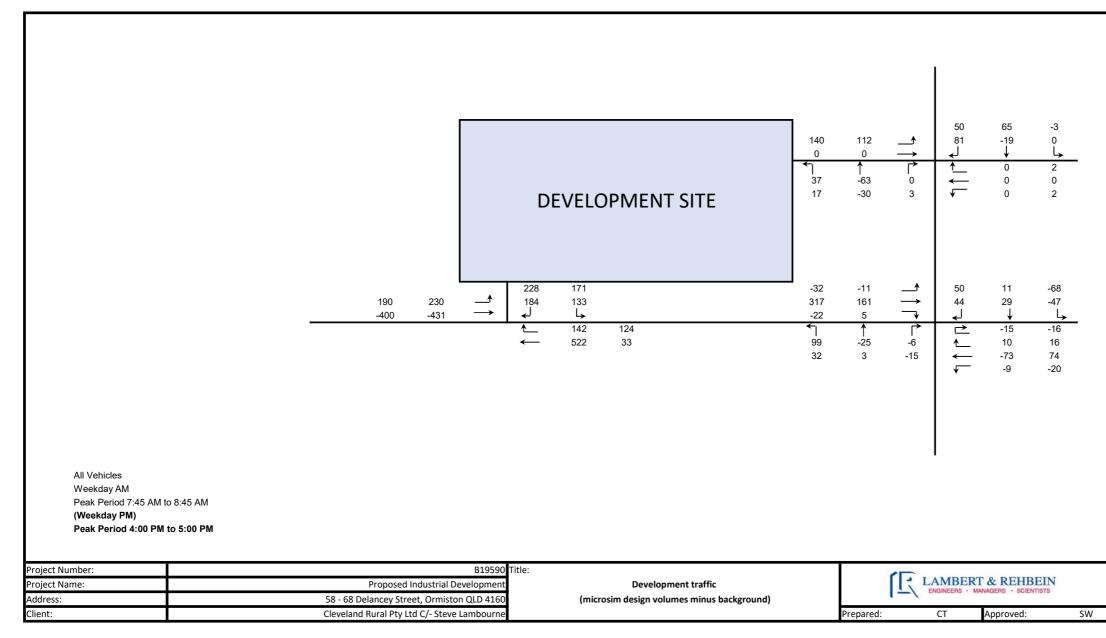


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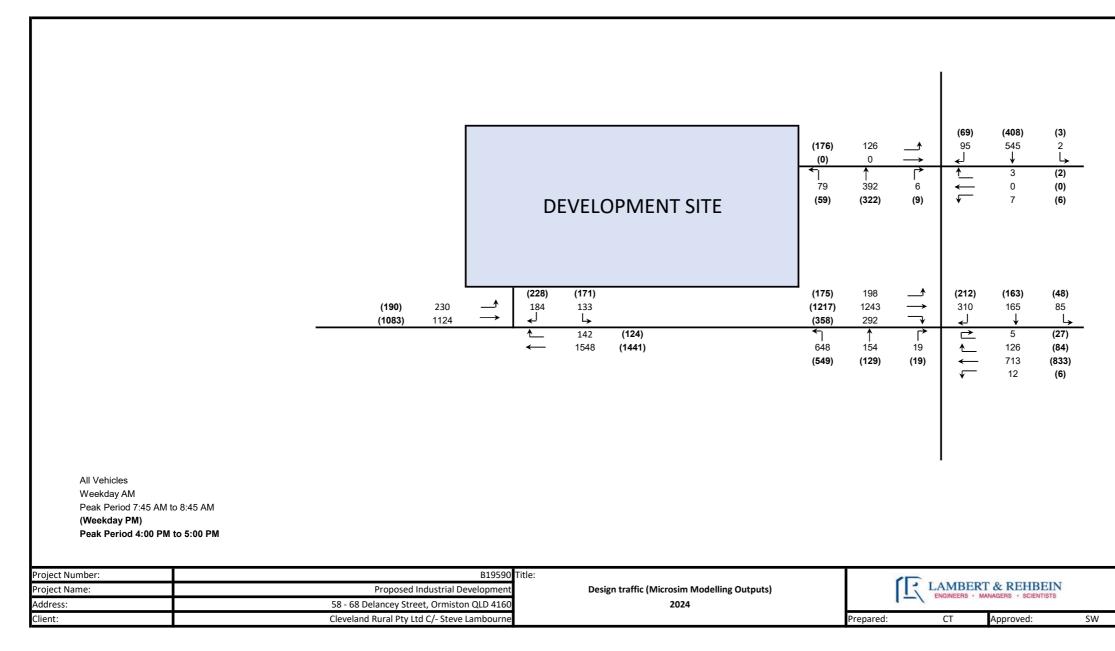


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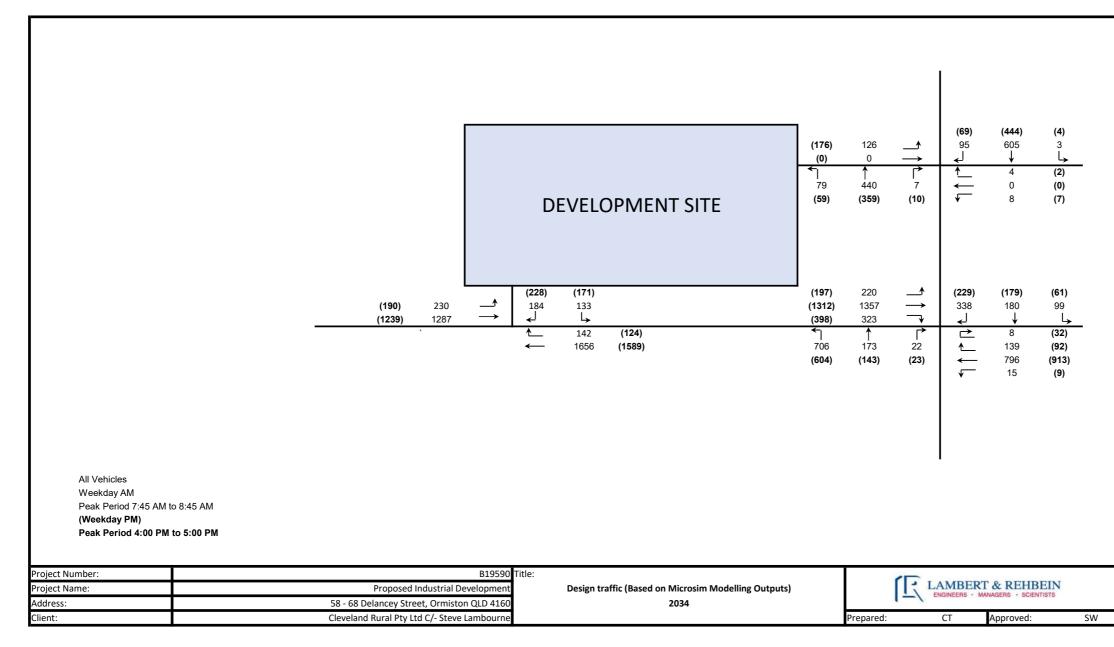


		Figure Number:
 A Rev:	7/11/2022 Date:	C6

APPENDIX D- MICRO SIMLUATION MODELLING REPORT

ą danner

Private Hospital Finucane Road – Access analysis

Micro-simulation modelling

Analysis of the traffic impacts of a proposed private hospital with access onto Finucane Road, Cleveland

Charles Reeler Version 1 8/07/2022

Prepared by Adanner Pty Ltd for [insert client name]

Version	Date	Author	Change Description
1	8/07/2022	Charles Reeler	DRAFT
2	8/07/2022	Charles Reeler	FINAL

Report version control

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1 BACKGROUND

A private hospital with mixed-use development has been proposed to be developed on the corner of Finucane Road and Delancey Street in Cleveland. There is an existing access on Delancey Street servicing the current development however with further development of this site this existing access will be inadequate for the predicted traffic demands.

A second access to the development site is therefore proposed on Finucane Road to meet the increased traffic demands to and from the site.

The purpose of this document is to provide the results of an assessment of the traffic impacts of proposed development at the new Finucane Road access, the impact of the development on the surrounding road network and to identify mitigating measures to address the increased traffic demands.

This report documents the findings of the micro-simulation modelling carried out for this investigation.

2 METHODOLOGY

2.1 Study Area

Figure 1 shows the site of the private hospital and proposed mixed development and the surrounding road network that has been included in the simulation modelling.

The study area extends from McDonald Road in the west to Wellington Street in the east and from Russel Street in the south to Freeth Street in the north. Seven intersections are included in the study are as shown in the figure below.

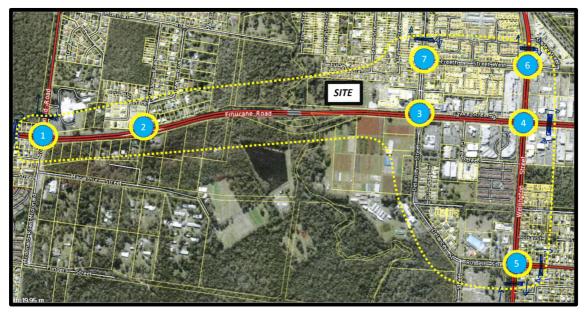


Figure 1 Study Area

The intersections included in the models are:

- 1. Finucane Road / McDonald Road
- 2. Finucane Road / Dawson Road
- 3. Finucane Road / Delancey Street
- 4. Finucane Road / Wellington Street
- 5. Wellington Street / Russel Street
- 6. Wellington Street / Freeth Street
- 7. Delancey Street / Freeth Street

The development site is located in the north west corner of the intersection of Finucane Road and Delancey Street. There is an existing access to the current site off Delancey Street opposite the Arthur Street intersection. The new second access is proposed off Finucane Road at the western end of the development site as shown below (site dimensions and access location – not to scale).



Figure 2 Site Location and Accesses

2.2 Proposed Land Use and Trip Generation

Previous studies investigating the proposed development and accesses have been carried out by Lambert & Rehbein (L&R). These studies include detailed trip generation rates for the development as well as assignment to and from the development site for the weekday AM and PM peak periods. These assumptions are documented in the L&R document titled: "Technical Note – Proposed Hospital ad Mixed Use Development, Cnr Finucane Road & Delancey Street, Old 4160, Access Intersection Analysis", dated 10/11/2021.

For the purposes of this analysis, the same trip generation and assignment assumptions have been assumed (albeit with slight changes as described in the following sections). In summary the proposed development mix is presented in the following table (extract from the L&R document):

USE	YIELD	
PRIVATE HOSPITAL	148 BEDS + 14 THEATRES	22,411 m ² GFA
DAY SURGERY	18 Day Beds	22,411 m GFA
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	-	4,614 m ² GFA
CHILD CARE	150 PLACES	1,200 m ² GFA
SPECIAL RETAIL	-	6,213 m ² GFA
RESEARCH INSTITUTE	-	4,407 m ² GFA
ASSISTED LIVING UNIT & FACILITIES	200 UNITS + FACILITIES	25,000 m ² GFA
AGED CARE	134 BEDS + Ancillary Area	11,163 m ² GFA
COMMUNITY HUB	-	2,000 m ² GFA
TOTAL		77,008 m ² GFA

Table 1 Proposed Development Yields

Based on these yields the trip generation for the development was estimated using the trip generation rates and directionality as shown in the following table.

LAND USE	GENERAT	ION RATE	DIRECTIONALIT	Y (% IN / % OUT)	SOURCE
	AM Peak	PM Peak	AM Peak	PM Peak	
PRIVATE HOSPITAL	-22.07 trips +1.04	-22.07 trips +1.04	50% in / 50% out	50% in / 50% out	DMS (formally DTA)
DAY SURGERY	trips x beds	trips x beds	50% in 7 50% out	50% III / 50% Out	RMS (formally RTA)
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	9.81 trips / 100m ² GFA	5.80 trips / 100m ² GFA	50% in / 50% out	50% in / 50% out	DTMR Open Source Data
CHILD CARE	0.8 trips / child	0.7 trips / child	50% in / 50% out	50% in / 50% out	RMS (formally RTA)
SPECIAL RETAIL	2.8 trips / 100m ² GFA	5.6 trips / 100m ² GFA	50% in / 50% out	50% in / 50% out	RMS (formally RTA)
RESEARCH INSTITUTE	2 trips / 100m ² GFA	2 trips / 100m ² GFA	90% in / 10% out	10% in / 90% out	RMS (formally RTA)
ASSISTED LIVING UNIT & FACILITIES	0.2 trips per dwelling	0.2 trips per dwelling	80% in / 20% out	20% in / 80% out	RMS (formally RTA)
AGED CARE	0.2 trips per dwelling	0.2 trips per dwelling	80% in / 20% out	20% in / 80% out	RMS (formally RTA)

Table 2 Trip Generation and Directionality Rates

With these laned uses and trip generation assumptions, the trip generation of the development was determined as follows:

LAND USE	YIELD	AM(IN)	AM (OUT)	PM (IN)	PM (OUT)
PRIVATE HOSPITAL	148 BEDS + 14 THEATRES	75	75	75	75
DAY SURGERY	18 Day Beds				
MEDICAL SPECIALISTS IN CONSULTING SUITES BLDG	4,614 m ² GFA	226	226	134	134
CHILD CARE	150 PLACES	60	60	53	53
SPECIAL RETAIL	6,213 m ² GFA	87	87	174	174
RESEARCH INSTITUTE	4,407 m ² GFA	79	9	9	79
ASSISTED LIVING UNIT & FACILITIES	200 UNITS + FACILITIES	32	8	8	32
AGED CARE	134 BEDS + Ancillary Area	21	5	5	21
COMMUNITY HUB	2,000 m ² GFA	-	-	-	-
Total Trips per Peak (10% Cros	ss-Utilisation)	524	424	412	512

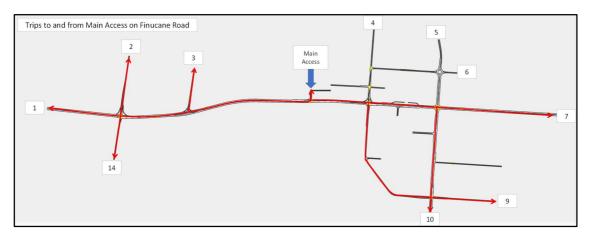
Table 3 Development Traffic Generation

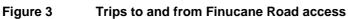
These trips were then assigned with the basic assumptions that the existing access on Delancey Road would attract 20% of the trips and the new Finucane Road access would attract the remaining 80% of the trips.

For the purposes of this micro-simulation analysis these assumptions were slightly modified.

The current access on Delancey Street allows all movements. With the proposed development the movements at this access will change to left-in, left-out and right-in only. No right-turns out of the site onto Delancey Street will be permitted. As a result, the assumption is that vehicles leaving the site and travelling south to Wellington Street and Russell Street will use the main access on Finucane Road.

It was also assumed that vehicles to and from the west on Finucane Road would all use the "Main" access on Finucane Road rather than travel past this access to the access on Delancey Street. Similarly, it was assumed that vehicles to and from Delancey Street (to the north) and Wellington Street (to the north) would all use the Delancey Street access rather than travel past this access to the Finucane Road access. These assumptions are illustrated in the following figures showing the routes to and from the two access points from the various trip generation zones.





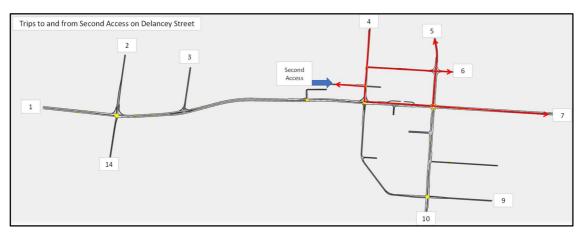


Figure 4 Trips to and from Delancey Street access

With these assumptions the original 80:20 split between the Finucane Road and Delancey Street accesses changed to 77:23.

Directional Split	Origin	al split	Reassigned split		
between accesses	AM	PM	AM	PM	
Finucane	80%	80%	77%	77%	
Delancey	20%	20%	23%	23%	

Table 4 Directional Split of Trips to and from the Development

The trip generation (OD matrices) for this reassignment is shown in more detail in Annexure A.

2.3 Simulation Model Network

As mentioned previously, the study area comprises

• Finucane Road from McDonald Road in the west to Wellington Street in the east

- Wellington Street from Russel Street in the south to Freeth Street in the north
- Delancey Street / Russel Street from Wellington Street in the south to Freeth Street in the north

There are 7 intersections in the network with 11 'external' trip generation zones. Additional zones were added to the network to act as sinks or generators to balance the trips arriving at and departing from the seven intersections where traffic count data is known.

In addition, an 8th intersection was added at the location of the proposed new access to the development on Finucane Road. The development site was also allocated two separate zones, one for the Finucane Road access and one for the Delancey Street access. The full simulation network showing the trip generation zones and the new access is presented in the following figure. Zones 15 and 18 are the zones allocated to the development.

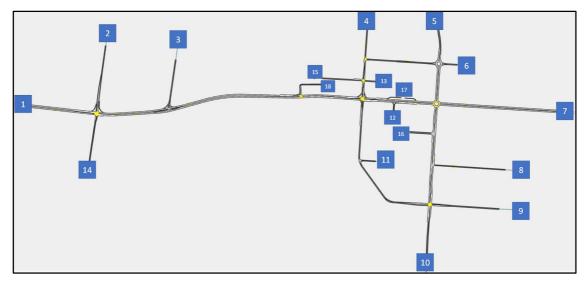


Figure 5 Simulation network and trip generation zones

2.4 Traffic Signal Data

Traffic signal data was provided for the following intersections:

- Finucane Road / McDonald Road
- Finucane Road / Delancey Street
- Wellington Street / Russell Street

The data provided comprised the signal phase sequences and actual timing data (length of time for each phase in May 2022). While the signal phasing data specifies the cycle lengths during the peak periods (160 sec AM peak and 130 sec PM Peak), the actual phase times data varies so that the actual cycle times varied and were not always equal to the stated cycle times.

For the purposes of the modelling, it was therefore assumed that the signals are isolated and operate under actuated control in the Base case models. The limits of the variability of the signal phases were then set to the minimums and maximums as recorded in the data provided.

2.5 Model Software

The simulation software used for this analysis is AIMSUN Next 20 (Version 20). (Note that using a different version of the software may result in slight changes to the results).

2.6 Calibration of 2022 Base Case Models

For this analysis, the Base Model is defined as the 2022 network as the calibration is based on the turn count data collected in March 2022.

The initial demand matrices were derived by assignment of trips between zones using a proportional distribution based on the volumes of attractions and generations to and from each zone. The models were then run to generate modelled turn flows at each intersection which were then compared to the count data. Based on these results, some manual adjustments were made to specific OD pairs so that an acceptable calibration could be achieved. The final manual adjustments included adding trips to and from the internal 'sink' zones. This iterative process was carried out until suitable calibration was achieved.

While route choices in the models are limited, there are certain route choices that required some minor adjustments to the default attributes in the software. Attribute overrides (additional delays) were applied at two locations, namely:

- In the AM Peak at the Wellington / Finucane roundabout (four movements)
- In the AM Peak at the Wellington / Russell intersection (two movements)
- In the PM peak at the Wellington / Finucane and the Wellington / Freeth roundabouts (four movements)
- In the AM Peak at the Wellington / Russell intersection (two movements)

In addition to these changes a number of OD routes were defined to prevent unrealistic route choices:

- Between Finucane east and Finucane west (both directions)
- From Finucane east to Wellington south
- From Finucane east to Russel Street

All models were run for 2 hours (one hour warm-up and the second hour used for all the result reporting). Five seed values were used to replicate the natural variability in daily traffic patterns and all results are based on the average of the five simulation runs. The best calibration was achieved using a proportional distribution whilst allowing on-route path updates using 5-minute calculation intervals.

The criteria for calibration of simulation models are typically based on the R-squared statistic and GEH values (comparison between actual intersection count data and the modelled turn volumes). The calibration criteria with the values from the models is shown in Table 2.

Calibration Criteria	Actual achieved			
Calibration Criteria	AM	PM		
R-squared > 0.9	0.987	0.979		
GEH < 5 in at least 85% of observations	93%	94%		

Table 5 Calibration Criteria

2.6.1 R-Squared

The R-squared statistic is presented in the following graphs showing the goodness of fit between the intersection counts and the modelled turn volumes for each peak period.

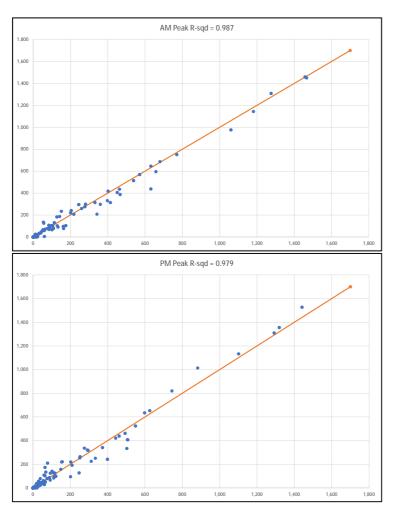


Figure 6 R-Squared Values for 2022 Base models

These graphs show that the models replicate the count data very well with the R-squared statistic well within the recommended limits.

2.6.2 Turn Volume Calibration

The tables below shows the GEH values for the turn volumes at the intersections for each peak hour.

				COUN	T Data	Modelled Volumes				
				AM Peak	PM Peak	AM	Peak	PM	Peak	
ntersection	Approach	Description	Turn	Total	Total	Total	GEH	Total	GEH	
			L	3	14	4	0.3	15	0.1	
ntersection 1 Finucane / McDonald 2 Finucane / Dawson	South	McDonald	Т	34	25	35	0.1	21	0.6	
	oouur	Webbilaid	R	105	88	97	0.5	87	0.1	
			U	AM Peak PM Peak Total Total						
			L						0.5	
	East	Finucane	Т						0.5	
4.53			R				0.3		0.4	
			U	-	-	-	0.5		0.0	
IVICDONAID			L						0.0	
	North	McDonald	I R						0.1	
			U				0.0		0.2	
			1	-			0.1	-	0.6	
			Т						0.0	
	West	Finucane	R						0.2	
			U						0.2	
			L	0				0		
	0		Т	-						
1 Finucane / McDonald 2 Finucane /	South		R	0		0		0		
			U	0	0	0		0		
			L	0	0	0		0		
	Fact	Finucane	Т	1,458	1,292	1,459	0.0	1,313	0.4	
	Lasi	Tinucane	R	84	110	107	1.7	129	1.2	
2 Finucane /			U	-						
Dawson			L	89	42	80	0.7	37	0.6	
	North	Dawson	Т	-	-			-		
			R	-			2.2		1.0	
			U							
		Finucane	L					-	4.6	
	West		T				0.3		1.8	
			R	-						
	South	Delancey	U	-	-	-	0.0	-	2.1	
			-	~					2.1	
			T R						1.2 1.9	
			U U	-			2.1		1.7	
			1	-			0.5		4.9	
		Finucane	Т	-				-	0.9	
	East		R						0.9	
3 Finucane /			U	-					1.7	
	-		L						2.3	
,	Manth	Dalaasaa	Т	133	149	91	2.8	162	0.7	
	North	Delancey	R	260	157	260	0.0	223	3.4	
			U	0	1	0		0	1.0	
			L	204	202	241	1.8	214	0.6	
	cane / South cane / North cane / South cane / South cane / South cane / South cane / North cane / North cane / North cane / South cane / South cane / South cane / South cane / South cane / South	Finucane	Т	1,060				1,008	2.9	
	** 651	riflucarie	R				0.8	PMP GEH Total 0.3 15 0.1 21 0.5 87 0.7 1,123 0.3 80 0.5 124 0.7 1,123 0.3 80 0 5 0.5 88 0.1 31 0.6 42 0 0 0.1 56 0.8 1,358 0.5 14 1.4 0 0 0 0 0 0.5 14 1.4 0 0 0 0 0 0 0 0.7 37 0 0 0.7 37 0 0 0.7 37 0 0 0.7 37 0 0 0.7 37 </td <td>1.4</td>	1.4	
			U							
	1		L						2.0	
	South	Wellington	T	7					6.7	
			R						2.3	
			U						1.0	
			L						1.7	
	East	Finucane	T						0.8	
4 Finucano /			R U						0.1	
									3.6	
wennigton			L T	-					0.8	
	North	Wellington	I R						3.6 1.6	
			к U	-					2.4	
			L	-					2.4	
			T	-					0.7	
	West	Finucane	R						2.9	
	VV CSL									

				COUN	T Data		Modelled	l Volumes	
				AM Peak	PM Peak	AM			Peak
ntersection	Approach	Description	Turn	Total	Total	Total	GEH	Total	GEH
			L	244	252	296			1.9
ttersection 5 Wellington / Russell 6 Wellington / Freeth 7 Delancey / Freeth 8 Delancey / Arthur			т	-	443				1.7
	South	Wellington	R						0.5
			U					1.0	
	Approach Description Turn Total Total	0.3							
	_		т	-					5.6
	East	Russell					AM PeakPMotalGEHTotal962.22971143.7395790.16001.00440.246972.4137030.1205000355.81826690.046624.33900013.951974.08624.377180.6526150.016000782.3771180.6526150.01600030.62400.7201.061000110.344460.4441580.1771300.623120.44312.5231265.1135000662.68901.00862.520872.7326000330.8901.0050.82000331.02345.9404140.311	7.5	
5 Wellington /									
			1	-			5.8		7.5
			т	-					0.9
	North	Wellington							2.7
			-	-					1.4
			1				30		2.2
			т	-					6.8
	West	Russell							0.9
				-			0.0		0.7
			-				22		2.7
									0.7
	South	Wellington		-					0.7
			-	10	10	10	0.0		0.4
				C	22	2	0.4		0.2
			_	-					0.2
	East	Freeth		-					
(Mallington (I	1	U	1.0		0.3
5				1	2	1	0.2		0.4
				-					0.4
	North	Wellington	-	-					0.7
		-	-	59	39	58	0.1		3.5
		Freeth I							3.6
	West			-					0.7
				151	63	235	4.3		1.8
				_				-	
		Delancev	-						
	South		-	-					3.5
							5.1		4.6
			U			-			1.0
				-			2.6		2.0
	Fast	Freeth			0	0		0	
Freeth	2001	1 room		-					1.6
			U	1	0	0	1.0	0	
			L	-					4.5
Wellington / Russell / Freeth / Freeth / Freeth / Freeth / Freeth / Ea Sou Wellington / Freeth / Ea Sou Sou Sou Sou Sou Sou Sou Sou Sou Sou	North	Delancev		-			2.7		1.2
	110/11/	Doranooy		-					
			-						
			L	41	27	37	0.5	29	0.2
	South	Delancev		463	503	AMPeak Total GEH 296 2.2 314 3.7 79 0.1 0 1.0 0 1.0 297 2.4 103 0.1 0 - 135 5.8 569 0.0 2 4.3 0 - 1 3.9 97 4.0 276 0.0 2 4.3 0 - 78 2.3 418 0.6 15 0.0 70 1.0 31 0.6 0 0.7 0 1.0 1 0.3 646 0.4 58 0.1 72 0.8 2 0.4 235 4.3 0 1.2 0 1.2 0 <td< td=""><td>337</td><td>5.7</td></td<>	337	5.7	
	Coun	Delanoey	R	5	5	3			1.0
							2.2 297 3.7 395 0.1 60 1.0 0 0.2 46 2.4 137 0.1 205 0 0 5.8 182 0.0 466 4.3 39 0 3.9 51 4.0 4.0 86 0.0 314 0 2.3 77 0.6 0.6 526 0.0 16 0 0 0.6 526 0.0 16 0 0 0.6 526 0.0 16 0 0 0.3 4 0.4 441 0.1 77 0 0 0.8 105 0.4 4 4.3 45 0 0	1.0	
			L	7	8	5	0.6	7	0.3
	Fact	Arthur		0	0	0		0	
	LdSI	Aittiui	R	3	1	5	0.8	2	0.5
8 Delancey /				0	0	0		0	
Arthur			L	1	7	3	1.0	2	1.6
	North	Delensor	Т	631	508	438	5.9	404	3.4
	INORTH	Delancey	R	12	15		0.3		0.8
			U	1	1				1.0
		1	L	14	39				1.6
			T	0	0				
	West	Site Access	R	13	29		0.8		2.6
			U	0	0				

Table 6 2022 Base model GEH Values for all turn movements

The focus of the calibration exercise was to replicate the turn volumes at the Finucane Road / Delancey Street intersection as this is the intersection likely to be the most affected by the

development traffic. GEH values of less than 5 (green cells) have been achieved for 93% and 94% of all turn movements in the AM and PM peak periods respectively. All the turn movements at the Finucane Road / Delancey Street intersection have GEH values of 5 or less. This is well within the recommended guidelines for calibration. More detailed calibration data including light and heavy vehicles volumes is included in Annexure B.

The 2022 Base models are therefore considered to be fit for purpose to be used to evaluate the impact of the proposed new access to the development on Finucane Road.

3 MODELLING RESULTS

3.1 Performance Criteria

The modelling evaluates the operation of the network and intersections during the weekday AM and PM peak periods. The count data indicates that the peak hours at the Finucane / Delancey intersection are as follows:

- AM Peak: 7:45am to 8:45am
- PM Peak: 4:00pm to 5:00 pm

These peak hours were therefore adopted for the models. The models were all run for 2 hours, one hour prior to the peak hour (warm-up period), and one hour representing the peak hour. Demands from zones on Finucane Road (east and west), McDonald Road, Russel Street and Wellington Street were profiled so that the releases into the network match the count data profiles.

All models were run with five seed values (RTA specified: 28, 560, 2859, 7771, 86524) and the reported results are the average for the five seeds.

The performance of the intersections is based on the modelled delays for all turn movements. The approach and intersection delays are a weighted average of the individual turn delays. The LOS criteria for signalized intersections, roundabouts and sign-controlled intersections are as per the following table (same criteria as used by SIDRA):

SIDRA LOS Criteria	Signal	Sign controlled	Roundabout
А	10	10	10
В	20	15	20
С	35	25	35
D	55	35	50
E	80	50	70
F	>80	>50	>70

Table 7 Level of Service Criteria (seconds)

3.2 Modelled Scenarios

To quantify the impact of the development traffic and the proposed upgrades to mitigate the impacts, the following scenarios were modelled:

Scenario	Network	Demands	Signals
1	Base (existing)	Background traffic	Existing
2	Base + development access	Background + development	Existing
3	Base + development access + Upgrade 1	Background + development	Revised for Upgrade 1
4	Base + development access + Upgrade 2	Background + development	Revised for Upgrade 2

Table 8 Modelled Scenarios

3.2.1 Scenario 1 – Existing Situation

These models represent the current operating conditions and serve as a base line or reference against which the other models are compared. As mentioned previously, the Base models used the signal phase times (as provided) and operate under actuated control. With these assumptions and signal control, a suitable calibration was achieved.

The Base model network in the vicinity of the development site is shown in the figure below.



Figure 7 Base Model Network

The most important intersection in the network is the Finucane Road / Delancey Street intersection and this intersection effectively controls the operation of the traffic through the

simulation network in the vicinity of the development. The signal phase sequence used in the Base models (as per the data provided) is as follows:



Figure 8 Finucane/Delancey Intersection - Base Model Signal Phase Sequence

The signal data provided and used in the models is contained in Annexure E.

3.2.2 Scenario 2 – Existing Network with Development Traffic

This scenario comprises the inclusion of the development traffic and the new access on Finucane Road to the development. No other changes are made to the road network or the signal operation at the Finucane Road / Delancey Street intersection. The purpose of this scenario is to illustrate the impact the development traffic would have on the road network without any upgrades.

The development access on Finucane Road has a separate right-turn into the development (signalised and only triggered on demand) and two exiting lanes from the development site, also only triggered on demand.



Figure 9 Base Network with Development Accesses

The layout of the Finucane Road access intersection is shown below.

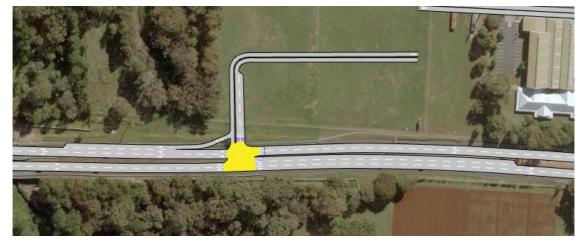


Figure 10 Access to Development site from Finucane Road

The signal phasing at this intersection is designed to minimise delays to the through traffic. The proposed signal phasing at the intersection is as follows:



Figure 11 Signal Phasing – Finucane Road Development Access

3.2.3 Scenario 3 – Upgrade 1

The simulation models showed that some delays can occur on the NB approach to the Finucane Road / Delancey Street intersection. Northbound through vehicles on Delancey Street can queue at the stop line of the intersection with queues extending south on Delancey Street to beyond the start of the left-turn slip-lane from Delancey Street to Finucane Road. When this happens, the vehicles turning left cannot get into the left-turn lane and are delayed. To reduce the delays for these vehicles, a potential upgrade would be to extend the left-turn slip-lane so that leftturning vehicles have the ability to get into the left-turn lane without being delayed behind the stopped through traffic. This is shown in the figure below.

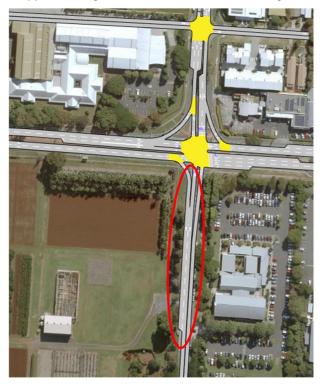


Figure 12 Upgrade Option 1 – Lengthened left-turn slip lane from Delancey Street to Finucane Road

In addition to these changes, the lane arrangement on the Delancey Street north approach was also altered by changing the lane configuration from a right-turn lane and a through lane to a right-turn lane and a shared through/right-turn lane.

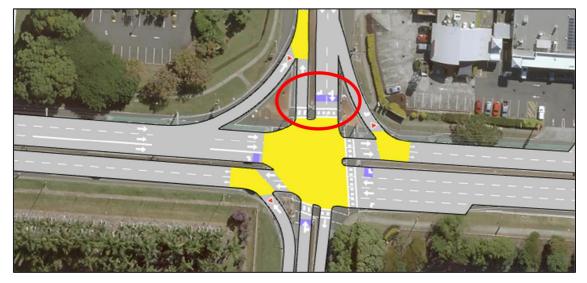


Figure 13 Finucane Road / Delancey Street - Lane arrangement

As the Finucane Road access intersection is relatively close to the Finucane Road / Delancey Street intersection (approximately 330m), the signals have been co-ordinated to encourage progression and to minimise delays. As a result, the signal phasing at the Finucane Road / Delancey Street intersection is no longer isolated but runs actuated and is co-ordinated with the signal at the development site access. The cycle times used are 160 seconds in the AM peak and 130 seconds in the PM peak (as these are the cycle times that are specified in the signal data received).

With this arrangement, it was found that some benefits could be achieved by some changes to the signal phasing at the Finucane Road / Delancey Street intersection by introducing a right-turn phase from Finucane Road EB to Delancey Street SB (last phase in the sequence below). (Note that this additional phase is already in the options for the signal currently in use however the data provided appears to indicate that this signal phase was not triggered).



Figure 14 Finucane/Delancey Intersection – Upgrade 1 Signal Phasing with Development

3.2.4 Scenario 4 – Upgrade 2

In this scenario a further upgrade to the Finucane Road / Delancey Street intersection is proposed comprising the addition of a second right-turn lane from Finucane Road EB to Delancey Street SB. This will require some widening of Delancey Street south of the intersection to accommodate the dual right-turn lanes. This "Upgrade 2" option is shown in the figure below.

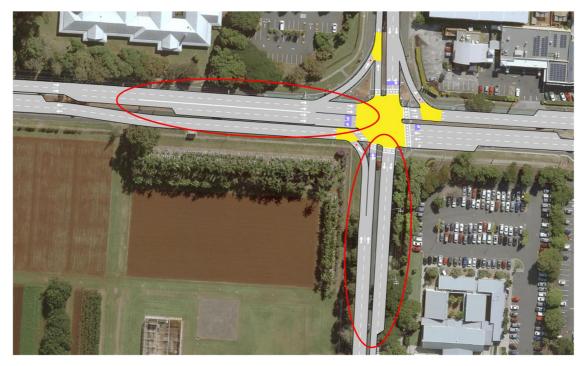


Figure 15 Upgrade Option 2 – Dual right-turn lane Finucane Road to Delancey Street

With this option the previous signal phases are not appropriate because the opposing rightturns (4th phase - EB to SB and WB to NB) cannot occur in the same phase. The signal phase sequence proposed for this option is as follows:

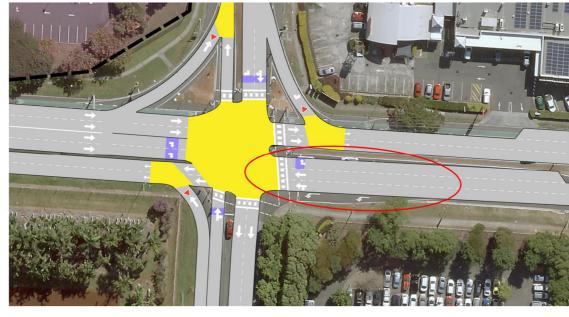


Figure 16 Finucane/Delancey Intersection – Upgrade 2 Signal Phasing with Development and dual right-turn lanes

The additional capacity provided by the second right-turn lane from Finucane Road means that more green time can be allocated to the east-west through movements as well as to the traffic on the Delancey Street north and south approaches to the intersection. This also encourages traffic to use Delancey Street to Wellington Street rather than proceeding east on Finucane Road to Wellington Street via the Wellington Street / Finucane Road roundabout which is the most congested during peak periods.

3.2.5 Scenario 5 – Upgrade 3

This scenario is similar to Scenario 4 but with a relatively minor change to the lane arrangement on Finucane Road east approach to the intersection. In this option the left-turn movement is combined with the through movement in the kerbside lane with a through lane and a right-turn lane.



This "Upgrade 3" option is shown in the figure below.

Figure 17 Upgrade Option 3 – Dual right-turn lane Finucane Road to Delancey Street

Changing this east approach may help facilitate providing the dual right-turn lanes on the west approach while minimising the construction impacts.

3.3 Intersection Performance

The performance of the network is based on the modelled delays at all the intersections in the network. From the model outputs the intersection level of service (LOS) can then be determined for each turn movement, each approach and each intersection. A summary of the intersection performance results using delay as the performance metric to determine the LOS is shown in the table below.

Intersection LOS			AM					PM		
Intersection LOS	Scenario 1	Scenario 2	Scenario 3	Scenario 4	Scenario 5	Scenario 1	Scenario 2	Scenario 3	Scenario 4	Scenario 5
Intersection	Base Network	Base + development	Base with development + Upgrade 1	Base with development + Upgrade 2	Base with development + Upgrade 2	Base Network	Base + development	Base with development + Upgrade 1	Base with development + Upgrade 2	Base with development + Upgrade 2
1 Finucane / McDonald	С	С	D	С	С	С	С	С	С	С
2 Finucane / Dawson	Α	Α	Α	А	Α	Α	А	Α	А	Α
3 Finucane / Delancey	E	E	E	D	D	E	E	D	С	С
4 Finucane / Wellington	С	D	С	С	С	В	С	С	С	С
5 Wellington / Russell	E	E	F	E	E	D	D	E	D	D
6 Wellington / Freeth	Α	А	А	А	Α	Α	А	А	А	A
7 Delancey / Freeth	E	F	F	С	С	Α	А	Α	А	Α
8 Delancey / Arthur	D	В	В	А	Α	E	А	Α	А	А
9 Finucane / Site Access		В	В	В	С		С	В	В	В

Table 9 Intersection Performance – Level of Service

The associated average vehicle delays for the LOS metrics are presented in the following table.

Intersection delays			AM					PM		
intersection delays	Scenario 1	Scenario 2	Scenario 3	Scenario 4	Scenario 5	Scenario 1	Scenario 2	Scenario 3	Scenario 4	Scenario 5
Intersection	Base Network	Base + development	Base with development + Upgrade 1	Base with development + Upgrade 2	Base with development + Upgrade 3	Base Network	Base + development	Base with development + Upgrade 1	Base with development + Upgrade 2	Base with development + Upgrade 2
1 Finucane / McDonald	32.8	34.6	35.5	34.2	33.7	31.8	30.7	30.9	30.6	31.0
2 Finucane / Dawson	3.5	3.6	3.1	3.1	3.2	3.8	4.0	3.2	3.3	3.4
3 Finucane / Delancey	60.2	76.1	58.0	48.6	49.5	61.9	68.3	35.7	32.0	33.4
4 Finucane / Wellington	23.4	38.0	29.1	31.9	24.7	17.7	26.4	24.4	21.2	20.6
5 Wellington / Russell	60.0	75.2		77.7	77.6	45.6	50.0	54.4	50.6	50.2
6 Wellington / Freeth	4.8	7.3	3.9	3.3	3.0	3.3	2.9	3.9	2.8	3.0
7 Delancey / Freeth	49.3	74.5	56.8	22.4	23.0	8.5	7.5	6.9	3.7	2.2
8 Delancey / Arthur	33.8	13.9	12.9	7.3	7.6	44.3	6.5	8.5	5.9	4.2
9 Finucane / Site Access		16.3	11.4	13.1	22.1		24.7	13.5	13.1	16.8

Table 10 Intersection Performance – Delay (seconds / vehicle)

These results indicate that with the identified intersection upgrades similar intersection performance and delays to the original background vehicles can be achieved even with the development traffic added to the overall traffic demands.

(While no upgrades at the Wellington Road / Russell Street intersection were tested, the model suggest that some relatively minor upgrades to this intersection could result in further network performance benefits).

Annexure C contains more detailed results showing the intersection performance metrics for each scenario.

3.4 Network Performance

An overall performance measure for the network can be defined as the sum of all the delays at all the intersections. This is simply the sum of each turn movement multiplied by the associated delay for that turn movement for all intersections in the network. This calculation produces a number that is indicative of the total delay for all intersections in the network.

As before, the Base network can be used as a point of reference to determine the impact the development will have on delays. TMR require mitigation measures to be implemented so that the vehicles in the original Base network, or original background traffic, is not adversely affected by the new development traffic and the associated increases in delays that these vehicles will cause.

To do this comparison, the base case for the background traffic is determined. This is the reference "delay number".

The background traffic turn volumes through all the intersections are multiplied by the "new" delays for each turn movement with the development traffic. Note that because some upgrades or changes to the network have been proposed, the routes taken by the original background traffic may have changed from their original routing to take advantage of the upgrades and improvements. For example, background traffic previously travelling from Finucane Road west to Wellington Street south may have travelled via Finucane Road through Finucane Road / Wellington Street roundabout. With the intersection upgrades at Delancey Street, some of these vehicles may choose to turn right into Delancey Street as this route option may be quicker.

With the simulation models this calculation is easily done by defining different vehicle types for the background traffic and the development traffic. The development traffic vehicles are then simply multiplied by the associated turn delays at all intersection to produce the new "delay number" for the upgraded network.

Dealaround Traffia (Sum of turn volume v delav)	T	otal Delays (s	sec)
Background Traffic (Sum of turn volume x delay)	AM	PM	Both peaks
Network Scenario	Total	Total	All traffic
Scenario 1: Base existing network	629062	532573	1161636
Scenario 2: Base existing network with development	805784	618853	1424637
Scenario 3: Development + Upgrade 1 (lengthen slip-lane)	691710	474789	1166498
Scenario 4: Development + Upgrade 2 (slip lane + dual rights)	615284	433747	1049031
Scenario 5: Development + Upgrade 3 (slip lane + dual rights + re-alignment)	615128	433418	1048546

The table below presents the results of these calculations.

Table 11 Background Traffic Total Delay (sum of vehs x delay)

These results show that with the development traffic, there could be some additional delays to the background traffic, particularly during the AM peak. With Upgrade 1 the delays are mitigated to some extent with the overall delay for both peaks somewhat less than the delays in the original Base network. However, the delays during the AM peak still exceed the AM peak delays in the Base network.

With Upgrade 2 the total delays to the background vehicles in both peaks are reduced when compared with the delays in the Base network. The percentage increase or decrease in delays is shown in the following table.

Background traffic % Increase in delay vs Base	AM	PM	Both peaks
Network Scenario	Total	Total	All traffic
Scenario 2: Base existing network with development	28.1%	16.2%	22.6%
Scenario 3: Development + Upgrade 1 (lengthen slip-lane)	10.0%	-10.9%	0.4%
Scenario 4: Development + Upgrade 2 (slip lane + dual rights)	-2.2%	-18.6%	-9.7%
Scenario 5: Development + Upgrade 3 (slip lane + dual rights + re-alignment)	-2.2%	-18.6%	-9.7%

Table 12 Percentage increase in delays to Background Traffic vs Existing Base Network Percentage increase in delays to Background Traffic vs Existing Base

4 CONCLUSIONS

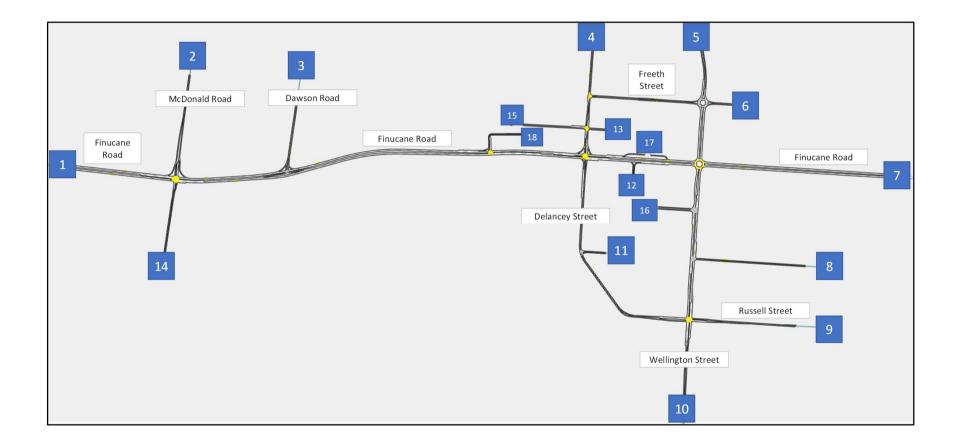
Based on the results of the simulation models, it is concluded that:

- The current network functions relatively well but some intersections, namely the Finucane Road / Delancey Street and the Wellington Road / Russell Street intersections, can be prone to congestion during peak periods.
- The development traffic will impact the performance of the network with the greatest impact likely to occur at the Finucane Road / Delancey Street intersection.
- With the development traffic added to the existing background traffic, delays can be expected to increase. However, these delays can largely be mitigated through some targeted network upgrades in conjunction with traffic signal operation changes.
- The current signal phasing will need to change so that the signals at the access to the development and the signals at the Finucane Road / Delancey Street intersection are vehicle actuated and coordinated with a common cycle time.
- The proposed upgrades are expected to encourage the use of the Delancey Steet connection between Finucane Road and Wellington Road. As a result, this is likely to impact the operation of the Wellington Road / Russell Street intersection which may require further localised upgrades to improve the operation at this location. (No evaluation of any upgrades at this intersection were investigated in this analysis)
- Upgrades 2 and 3 are expected to provide the best result in terms of reduced delays to the existing background traffic in both peak periods due to the significant increased capacity at the Finucane Road / Delancey Street intersection.
- There is little difference between Upgrades 2 and 3 (Scenarios 4 and 5) as the left-turn demand from Finucane Road WB to Delancey Street SB is very low and can easily be combined with the WB through traffic without a significant impact on the intersection performance.

ANNEXURE A – 2022 DEMAND MATRICES (WEEKDAY AM AND PM PEAKS)

This Annexure shows the original trip generation to and from the development site and the change in the assumptions regarding the use of the accesses.

Simulation Model Zones



AM Light	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	Total
1	0	30	0	90	138	6	495	0	58	318	0	0	1	9	13	0	0	1158
2	47	0	0	20	0	0	110	0	13	71	0	0	0	30	0	0	0	291
3	25	0	0	0	0	0	38	0	4	24	0	0	0	3	0	0	0	94
4	239	0	0	0	0	0	0	0	42	229	50	0	1	0	9	0	0	570
5	164	0	0	0	0	3	247	70	29	159	0	0	0	0	0	0	0	672
6	1	0	0	0	0	0	2	0	0	1	0	0	0	0	0	0	0	4
7	369	29	44	101	155	7	0	0	65	0	0	0	2	43	15	0	0	830
8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9	206	16	25	56	86	0	0	0	0	40	0	0	1	24	8	0	0	462
10	250	19	30	68	105	5	80	0	75	0	0	0	1	29	10	0	0	672
11	0	0	0	50	0	0	0	0	0	0	0	0	0	0	0	0	0	50
12	40	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	40
13	2	0	0	1	0	0	4	0	0	2	0	0	0	0	0	0	0	9
14	3	33	0	10	0	0	53	0	6	34	0	0	0	0	0	0	0	139
15	7	0	0	2	0	0	10	0	1	6	0	0	0	0	0	0	0	26
16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1353	127	99	398	484	21	1039	70	293	884	50	0	6	138	55	0	0	5017
AM Heavy	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	Total
AM Heavy 1	1 0	2	3 0	4 6	5 4	6 0	7 19	8 0	9 3	10 19	11 0	12	13 0	14 2	15 0	16 0	17	Total 54
																		Total 54 5
1	0	1	0	6	4	0	19	0	3	19	0	0	0	2	0	0	0	54
1 2	0 1	1 0	0	6 0	4	0	19 1	0	3 0	19 1	0	0	0	2 2	0	0	0	54 5
1 2 3	0 1 1	1 0 0	0 0 0	6 0 0	4 0 0	0 0 0	19 1 1	0 0 0	3 0 0	19 1 1	0 0 0	0 0 0	0 0 0	2 2 0	0 0 0	0 0 0	0 0 0	54 5 3
1 2 3 4 5 6	0 1 1 5	1 0 0	0 0 0	6 0 0	4 0 0 0 0 0	0 0 0	19 1 1 0	0 0 0	3 0 0 1	19 1 1 6	0 0 0	0 0 0	0 0 0	2 2 0 0	0 0 0	0 0 0	0 0 0	54 5 3 12 12 0
1 2 3 4 5 6 7	0 1 1 5 3	1 0 0 0 0 0 1	0 0 0 0 0 2	6 0 0 0 0 0 4	4 0 0 0 0 0 3	0 0 0 0 0 0	19 1 0 4 0 0	0 0 0 0	3 0 0 1 1	19 1 6 4 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	2 2 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	54 5 3 12 12 0 24
1 2 3 4 5 6	0 1 5 3 0	1 0 0 0 0	0 0 0 0 0	6 0 0 0 0	4 0 0 0 0 0	0 0 0 0 0	19 1 1 0 4 0	0 0 0 0 0	3 0 1 1 0	19 1 1 6 4 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	2 2 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	54 5 3 12 12 0
1 2 3 4 5 6 7	0 1 5 3 0 11	1 0 0 0 0 0 1	0 0 0 0 0 2	6 0 0 0 0 0 4	4 0 0 0 0 0 3	0 0 0 0 0 0	19 1 0 4 0 0	0 0 0 0 0 0	3 0 1 1 0 2	19 1 6 4 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	2 2 0 0 0 0 1	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	54 5 3 12 12 0 24
1 2 3 4 5 6 7 8	0 1 5 3 0 11 0	1 0 0 0 0 1 0	0 0 0 0 0 2 0	6 0 0 0 0 4 0	4 0 0 0 0 3 0	0 0 0 0 0 0 0	19 1 0 4 0 0 0 0	0 0 0 0 0 0 0	3 0 1 1 0 2 0	19 1 6 4 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	2 2 0 0 0 0 1 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	54 5 3 12 12 0 24 0
1 2 3 4 5 6 7 8 9 10 11	0 1 5 3 0 11 0 2 10 0	1 0 0 0 1 0 0 1 0 0	0 0 0 0 0 2 0 0 0 2 0 0	6 0 0 0 4 0 1 3 0	4 0 0 0 0 3 0 1 3 0	0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 4 0 0 0 3 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 0 2 0 0 2 0 0	19 1 6 4 0 0 2 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	2 0 0 0 1 0 0 1 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 12 0 24 0 9 22 0
1 2 3 4 5 6 7 8 9 10 11 12	0 1 5 3 0 11 0 2 10 0 0	1 0 0 0 1 0 0 1 0 0 0	0 0 0 0 2 0 0 2 0 0 0 2 0 0	6 0 0 0 4 0 1 3 0 0	4 0 0 0 0 3 3 0 1 3 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 4 0 0 0 3 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 0 2 0 0 2 0 0 0 0	19 1 6 4 0 0 0 2 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 0 0 1 0 1 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 12 0 24 0 9 22 0 0 0
1 2 3 4 5 6 7 8 9 10 11	0 1 5 3 0 11 0 2 10 0	1 0 0 0 1 0 0 1 0 0	0 0 0 0 0 2 0 0 0 2 0 0	6 0 0 0 4 0 1 3 0	4 0 0 0 0 3 0 1 3 0	0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 4 0 0 0 3 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 0 2 0 0 2 0 0	19 1 6 4 0 0 2 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	2 0 0 0 1 0 0 1 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 0 24 0 9 22 0 0 0 0 0
1 2 3 4 5 6 7 8 9 10 11 11 12 13 14	0 1 5 3 0 11 0 2 10 0 0 0 0 1	1 0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 2 0 0 2 0 0 0 0 0 0 0 0	6 0 0 0 0 4 0 1 3 0 0 0 0 0 0	4 0 0 0 0 3 0 0 1 3 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 4 0 0 3 0 0 0 1 1 1 1 0 0 0 0 0 0 1	0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 0 2 0 0 0 2 0 0 0 0 0 0	19 1 6 4 0 0 2 0 0 0 0 0 0 0 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 2 0 0 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 0 24 0 9 22 0 0 0 3
1 2 3 4 5 6 7 8 9 10 11 11 12 13 14 15	0 1 5 3 0 11 0 2 10 0 0 0 0	1 0 0 0 0 1 0 0 0 0 0 0 0	0 0 0 0 0 2 0 0 0 0 0 0 0 0	6 0 0 0 4 0 1 3 0 0 0 0	4 0 0 0 0 3 3 0 1 3 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 4 0 0 0 3 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 0 2 0 0 2 0 0 0 0 0 0	19 1 6 4 0 0 2 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 2 0 0 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 0 24 0 9 22 0 0 0 0 0
1 2 3 4 5 6 7 7 8 9 10 11 12 13 14 15 16	0 1 5 3 0 11 0 2 10 0 0 0 0 1 0 0 0	1 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0	0 0 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0	6 0 0 0 0 4 0 1 3 0 0 0 0 0 0 0 0 0	4 0 0 0 0 0 0 1 3 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 2 0 0 0 2 0 0 0 0 0 0 0 0 0	19 1 1 6 4 0 0 2 0 0 0 0 0 0 0 0 1 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 2 0 0 0 1 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 0 24 0 9 22 0 0 0 0 0 3 0 0 0 0 0 0 0
1 2 3 4 5 6 7 8 9 10 11 11 12 13 14 15	0 1 5 3 0 11 0 2 10 0 0 0 0 1 0	1 0 0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 2 0 0 0 2 0 0 0 0 0 0 0 0 0	6 0 0 0 0 4 4 0 1 3 0 0 0 0 0 0 0	4 0 0 0 0 3 3 0 1 3 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	19 1 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 1 1 2 0 0 2 0 0 0 0 0 0 0 0	19 1 1 6 4 0 0 0 2 0 0 0 0 0 0 1 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 2 0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	54 5 3 12 0 24 0 9 22 0 0 0 0 3 0 0

Simulation Model Background Traffic Demands – 2022 AM Peak

PM Light	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	Total
1	0	59	0	73	234	8	479	0	95	408	0	0	4	13	14	0	0	1387
2	44	0	0	6	0	0	40	0	8	34	0	0	0	32	0	0	0	164
3	16	0	0	0	0	0	17	0	3	14	0	0	0	2	0	0	0	52
4	118	0	0	0	0	0	0	0	24	104	100	0	1	0	4	0	0	351
5	162	0	0	0	0	3	167	0	33	142	0	0	0	0	0	0	0	507
6	9	0	0	0	7	0	9	0	0	8	0	0	0	0	0	0	0	33
7	411	41	68	65	208	7	0	0	84	0	0	0	4	63	12	0	0	963
8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9	101	10	17	16	51	0	105	0	0	41	0	0	1	15	3	0	0	360
10	238	24	39	38	120	4	0	0	55	0	0	70	2	36	7	100	0	733
11	0	0	0	100	0	0	0	0	0	0	0	30	0	0	0	0	0	130
12	0	0	0	0	0	0	40	0	0	0	0	0	0	0	0	0	0	40
13	3	0	0	0	0	0	3	0	1	2	0	0	0	0	0	0	0	9
14	12	25	0	6	0	0	41	0	8	35	0	0	0	0	0	0	0	127
15	20	0	0	3	0	0	21	0	4	18	0	0	0	0	0	0	0	66
16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1134	159	124	307	620	22	922	0	315	806	100	100	12	161	40	100	0	4922
PM Heavy	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	Total
1	0					0	1	0	9	10		14	13					
				0	1	0	6	0	0	6	0	0	0					
2		4	0	0	1	0	6	0	0	6	0	0	0	0	0	0	0	17
2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17 0
3	0	0	0	0	0	0	0	0	0	0	0	0	0	0 0 0	0 0 0	0 0 0	0 0 0	17 0 0
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17 0
3 4	0 0 2	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 2	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0 0	0 0 0 0	0 0 0	17 0 0 4
3 4 5	0 0 2 3	0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 3	0 0 0 0	0 0 0 0	0 0 2 3	0 0 0 0	0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	17 0 0 4 9
3 4 5 6	0 0 2 3 0	0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 3 0	0 0 0 0 0	0 0 0 0 0	0 0 2 3 0	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	17 0 0 4 9 0
3 4 5 6 7	0 0 2 3 0 11	0 0 0 0 0	0 0 0 0 0 4	0 0 0 0 0 1	0 0 0 0 0 2	0 0 0 0 0	0 0 3 0 0	0 0 0 0 0	0 0 0 0 0 1	0 0 2 3 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0 1	0 0 0 0 0 0 0	0 0 0 0 0 0	17 0 4 9 0 20
3 4 5 6 7 8	0 0 2 3 0 11 0	0 0 0 0 0 0	0 0 0 0 4 0	0 0 0 0 1 0	0 0 0 0 0 2 0	0 0 0 0 0 0	0 0 3 0 0 0	0 0 0 0 0 0 0	0 0 0 0 1 0	0 0 2 3 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 1 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	17 0 4 9 0 20 0
3 4 5 6 7 8 9	0 0 2 3 0 11 0 2	0 0 0 0 0 0 0	0 0 0 0 4 0 1	0 0 0 0 1 0 0	0 0 0 0 2 0 0	0 0 0 0 0 0 0	0 0 3 0 0 0 2	0 0 0 0 0 0 0 0	0 0 0 0 1 0 0	0 0 2 3 0 0 0 2	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 1 0 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7
3 4 5 6 7 8 9 10 11 12	0 2 3 0 11 0 2 5 0 0	0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 4 0 1 2 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0	0 0 0 0 2 0 0 0 1 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 3 0 0 0 2 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 1 0 0 0 0 0 0 0	0 0 2 3 0 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7 8 0 0 0 0
3 4 5 6 7 8 9 10 11	0 0 2 3 0 11 0 2 5 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 4 0 1 2 0	0 0 0 0 1 0 0 0 0 0	0 0 0 0 2 0 0 0 1 0	0 0 0 0 0 0 0 0 0 0	0 0 3 0 0 0 2 0 0 0	0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0	0 0 2 3 0 0 0 2 0 0 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 1 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7 8 0
3 4 5 6 7 8 9 10 11 12	0 2 3 0 11 0 2 5 0 0	0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 4 0 1 2 0 0 0	0 0 0 0 1 0 0 0 0 0 0	0 0 0 0 2 0 0 0 1 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 3 0 0 0 2 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 1 0 0 0 0 0 0 0	0 0 2 3 0 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7 8 0 0 0 0
3 4 5 6 7 8 9 10 11 12 13	0 0 2 3 0 11 0 2 5 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 4 0 1 2 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0	0 0 0 0 2 0 0 1 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 3 0 0 0 2 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0	0 0 2 3 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7 8 0 0 0 0 0
3 4 5 6 7 8 9 10 11 12 13 14	0 0 2 3 0 11 0 2 5 0 0 0 0 2	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 4 0 1 2 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 2 0 0 1 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 3 0 0 0 2 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 2 3 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7 8 0 0 0 0 2
3 4 5 6 7 8 9 10 11 12 13 14 15	0 0 2 3 0 11 0 2 5 0 0 0 0 0 2 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 4 0 1 2 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 2 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 3 0 0 0 2 0 0 0 0 0 0 0 0 0 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0	0 0 2 3 0 0 0 2 0 0 0 0 0 0 0 0 0 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	17 0 4 9 0 20 0 7 8 0 0 0 0 2 2 3

Simulation Model Background Traffic Demands – 2022 PM Peak

Original																				Revised																			
AM Peak																				AM Peak																			
eid	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15		17	18	Total	eid	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	Total
1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	36	0	0	145	181	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	181	181
2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	27	34	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	34	34
3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	11	14	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	14	14
4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	40	50	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	50	0	0	0	50
5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	15	19	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	0	0	0	19
6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	9	11	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	0	11
7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	0	0	63	79	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	0	0	63	79
8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	46	58	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	46	58
10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	48	60	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	48	60
11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	v	•	0	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13 14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0 0	0		0	0	0	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0 0	0
15	32	2	2	12 0	2	2	15 0	0	0	13 0	0	0	0	0	0	0	0	0	0	15		0	0	0			15	0	0	0	0	0	0	0	0	0	0	0	95
16 17	0	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16 17	0	0	0 0	0	0 0	0 0	0 0	0	0	0	0	0	0	0	0	0	0	0	0 0
17	128		9	49	8	7	60	0	12	50	0	0	0	0	0	0	0	0	0	17	160		11		0	0	60	0	15	63	0	0	0	0	0	0	0	0	317
Total	<u></u>	8		49 61	10	9	75	U		63	0	0	0	0	0	0	0	0	918	Total	160		11	61	10	9	75	0	15	63	0	0	0	0	119	-	0	387	918
Original	100	0		01	10	9	75		10	03	U	0	0	0	U	0	U	U	910	Revised	100	0	11	01	10	9	75	U	15	03	0	0	U	0	119	0	U	307	910
PM Peak																				PM Peak																			
eid	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	Total	eid	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	Total
1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	33	0	0	134	167	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	167	167
2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	9	11	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	11
3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	5	6	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	6
4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	29	36	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	36	0	0	0	36
5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	10	13	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	0	13
6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	7	9	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0	0	0	9
7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	53	66	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	53	66
8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	30	37	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	30	37
10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	43	54	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	43	54
11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15	37	2	4	14	3	3	16	0	5	15	0	0	0	0	0	0	0	0	79	15	0	0	0	69	14	16	16	0	0	0	0	0	0	0	0	0	0	0	115
16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18	149	10	14	55	11	13	64	0	22	61	0	0	0	0	0	0	0	0	320	18	186	12	18	0	0	0	64	0	27	76	0	0	0	0	0	0	0	0	383
Total	186	12	18	69	14	16	80		27	76	0	0	0	0	99	0	0	399	897	Total	186	12	18	69	14	16	80	0	27	76	0	0	0	0	89	0	0	310	897

Simulation Model Traffic Demands – Development Traffic Only

ANNEXURE B – INTERSECTION TURN VOLUME CALIBRATION

				AM Peak	COUN	T Data PM Peak			AM	Modelle	d Volumes	DA.4	Peak
Intersection	Approach	Description	Turn	Total	Light	PM Peak Heavy	Total	Light	AM Heavy	Total	GEH	Total	GEH
	South	McDonald	T	3 34	12 25	2	14 25	3	1	4	0.3	15 21	0.1
	douin	moboridia	R U	105 0	88 0	0	88	96 0	1	97 0	0.5	87 0	0.1
	_	_	L T	98 1,276	117 1077	0 24	117 1,101	103 1275	2 34	105 1,309	0.5	124 1,123	0.5
1 Finucane /	East	Finucane	R	62 0	75 0	0	75 0	62 0	3	65 0	0.3	80 0	0.4
McDonald			L	218	87	0	87	206	1	207	0.5	88	0.0
	North	McDonald	R	32 47	32 44	0	32 44	29 52	3	31 53	0.1	31 42	0.1
			L	0 31	0	0	0 63	0 31	0	0 32	0.1	0	0.6
	West	Finucane	T R	1,181	1305 13	14 0	1,319 13	1097	47	1,144	0.8	1,358	0.7
			U	2	0	0	0	0	0	0	1.4	0	
	South		T	0	0	0	0	0	0	0		0	
			R U	0	0	0	0	0	0	0		0	
	East	Finucane	L T	0 1,458	0 1268	0 24	0 1,292	0 1421	0 38	0 1,459	0.0	0 1,313	0.4
2 Finucane /	Lust	T Indeance	R	84 0	104	6	110 0	103 0	5	107 0	1.7	129 0	1.2
Dawson			L	89 0	41	1	42 0	76 0	4	80 0	0.7	37 0	0.6
	North	Dawson	R	12	12	0	12	24	1	25 0	2.2	17	1.0
			L	16	21	0	21	0	0	0	4.0	0	4.6
	West	Finucane	T R	1,466 0	1429 0	12 0	1,441 0	1402 0	49 0	1,451 0	0.3	1,538 0	1.8
			UL	0 538	0 496	0	0 506	0 514	0	0 514	0.8	0 441	2.1
	South	Delancey	T R	175	123 33	0	123 33	104	0	104 11	4.2	105 19	1.2
			U	0	0	0	0	0	0	0		0	
	East	Finucane	T	20 770	24 728	1	25 744	23 714	0 37	23 751	0.5	0 778	4.9
3 Finucane /			R U	113 19	65 42	1	66 42	126 0	4	130 0	1.1 4.4	56 28	0.9
Delancey			L	129 133	113 149	0	113 149	100 91	3	103 91	1.7	81 162	2.3 0.7
	North	Delancey	R	260	154	3	157	255	5	260	0.0	223	3.4
			L	204	200	2	202	230	12	241	1.8	214	0.6
	West	Finucane	T R	1,060 281	873 370	9 2	882 372	936 300	40 0	976 300	1.9 0.8	1,008 336	2.9 1.4
			UL	0 163	0 150	0	0 153	0 64	0	0 78	5.5	0 190	2.0
	South	Wellington	T	342 83	393 92	6	399 93	201 75	7	208 77	5.7 0.5	230 127	6.7 2.3
			U	3	1 107	0 4	1	0 64	0	0 66	1.7	0 87	1.0
	East	Finucane	T	658	611	14	625	579	17	596	1.7	654	0.8
4 Finucane /			R U	201 6	248 13	2	250 13	216 0	4	221 0	1.0 2.4	252 0	0.1 3.6
Wellington			L	245 451	206 327	3	209 334	290 395	5	296 405	2.2	192 248	0.8
	North	Wellington	R	127	90 6	2	92 6	178 0	5	182 0	3.1 2.6	71	1.6 2.4
			L	85	102	0	102	81	4	84	0.0	138	2.3
	West	Finucane	R	680 331	590 272	8	598 275	673 288	14 26	687 314	0.2	624 348	0.7
			UL	60 244	54 250	1 2	55 252	5 296	0	5 296	6.8 2.2	19 297	4.2
	South	Wellington	T R	414	436 55	7	443 55	292 76	22	314 79	3.7 0.1	395 60	1.7
			U	1 42	1 41	0	1 43	0 42	0	0	1.0	0 46	1.0 0.3
	East	Russell	T	360	242	5	247	297	0	297	2.4	137	5.6
5 Wellington /			R U	104 0	78 0	0	78 0	96 0	7	103 0	0.1	205 0	7.5
Russell	North	Wollington	L T	55 571	63 482	1	64 493	129 538	7 31	135 569	5.8 0.0	182 466	7.5
	North	Wellington	R	23	19	0	19	2	0	2	4.3	39	2.7
			L	19 161	30 200	1	31 201	1 97	0	1 97	3.9 4.0	51 86	2.2
	West	Russell	R	277	289	2	291	276	0	276	0.0	314	0.9
			UL	0 109	0 114	0	0 115	0 75	0	0 78	2.3	0 77	2.7
	South	Wellington	T R	402 15	544 18	5	549 18	407 15	11 0	418 15	0.6	526 16	0.7
			UL	2	23	0	23	3	0	3	0.6	0 24	0.2
	East	Freeth	T R	1	2	0	2	0	0	0	0.7	2	0.2
6 Wellington /			U									0	
Freeth	North	Wellington	T	1 631	3 452	0	3 461	1 633	0 13	1 646	0.3	4	0.4
			R U	59	39	0	39	58	0	58	0.1	77	3.5
			L	63 3	59 2	0	59 2	69 2	2	72	0.8	105 4	3.6 0.7
	West	Freeth	R	151	61	2	63	228	7	235	4.3	45 0	1.8
			L	0	0	0	0	0	0	0	0.5	0	0.5
	South	Delancey	R	398 57	310 68	2	312 69	320 122	11	331 126	2.5 5.1	231 135	3.5 4.6
			L	0	0 64	1	1 64	0 66	0	0 66	2.6	0 89	1.0 2.0
7 Delancey / Freeth	East	Freeth	T R	0 84	0 89	0	0 89	0 66	0	0 69	1.2	0 69	1.6
			U	1 142	0 60	0	0	0	0	0	1.0	0 20	4.5
Heedi			т	466	292	3	295	380	7	387	2.5	326	1.2
Heeu	North	Delancey			0	. 0	0	0	0	0		0	
Treeur	North	Delancey	R U	0	0	0	0	0	0	0		0	_
neeu				0 41	0 26	0 1 1	27	37	0 0 15	37	0.5	29	0.2
	North	Delancey Delancey	U L T R	0 41 463 5	0 26 502 5	1 1 0	27 503 5	37 421 3	0 15 0	37 436 3	0.9	29 337 9	5.7 1.0
	South	Delancey	U L T	0 41 463 5 1 7	0 26 502 5 1 8	1 1 0 0 0	27 503 5 1 8	37 421 3 0 5	0 15 0 0 0	37 436 3 0 5	0.9	29 337 9 0 7	5.7
			U T R U L T R	0 41 463 5 1 7 0 3	0 26 502 5 1 8 0 1	1 0 0 0 0 0	27 503 5 1 8 0 1	37 421 3 0 5 0 5	0 15 0 0 0 0	37 436 3 0 5 0 5	0.9 0.8 1.0	29 337 9 0 7 0 2	5.7 1.0 1.0
8 Delancey / Arthur	South	Delancey	U T R U L T	0 41 463 5 1 7 0 3 0 1	0 26 502 5 1 8 0 1 0 7	1 0 0 0 0 0 0 0 0	27 503 5 1 8 0 1 0 7	37 421 3 0 5 0 5 0 3	0 15 0 0 0 0 0 0 0 0	37 436 3 0 5 0 5 0 3	0.9 0.8 1.0 0.6 0.8 1.0	29 337 9 0 7 0 2 0 2	5.7 1.0 1.0 0.3 0.5 1.6
8 Delancey /	South	Delancey	U T R U L T R	0 41 5 1 7 0 3 0 1 631	0 26 502 5 1 8 0 1 0 7 501	1 0 0 0 0 0 0 0	27 503 5 1 8 0 1 0 7 508	37 421 3 0 5 0 5 0 5 0	0 15 0 0 0 0 0 0	37 436 3 0 5 0 5 0 5 0	0.9 0.8 1.0 0.6 0.8 1.0 5.9	29 337 9 0 7 0 2 0	5.7 1.0 1.0 0.3 0.5
8 Delancey /	South	Delancey Arthur	U T R U L T R U L T	0 41 463 5 1 7 0 3 0 1 631 12 1	0 26 502 5 1 8 0 1 0 7 501 14 1	1 0 0 0 0 0 0 0 7 1 0	27 503 5 1 8 0 1 0 7 508 15 1	37 421 3 0 5 0 5 0 3 431 14 0	0 15 0 0 0 0 0 0 0 8 0 0 0	37 436 3 0 5 0 5 0 3 438 14 0	0.9 0.8 1.0 0.6 0.8 1.0 5.9 0.3 1.0	29 337 9 0 7 0 2 0 2 404 11 0	5.7 1.0 1.0 0.3 0.5 1.6 3.4 0.8 1.0
8 Delancey /	South	Delancey Arthur	U T R U L T R U L T	0 41 463 5 1 7 0 3 0 1 631 12	0 26 502 5 1 8 0 1 0 7 501 14	1 0 0 0 0 0 0 7 1	27 503 5 1 8 0 1 0 7 508 15	37 421 3 0 5 0 5 0 3 431 14	0 15 0 0 0 0 0 0 8 0	37 436 3 0 5 0 5 0 3 438 14	0.9 0.8 1.0 0.6 0.8 1.0 5.9 0.3	29 337 9 0 7 0 2 0 2 404 11	5.7 1.0 1.0 0.3 0.5 1.6 3.4 0.8

ANNEXURE C – DETAILED INTERSECTION PERFORMANCE METRICS

The following tables contain the modelled turn volumes (background plus development traffic) and the associated delays for each turn movement. The approach and intersection delays are the weighted average of the respective turn delays.

							Modelled	Volumes									Backg	ground Traffic - W	Vithout Devel	opment						
Intersection	Approach	Description	Turr	Light	A	M Peak Heavy	Total	Light	PM Peak Heavy	Total	Light	Heavy	Turn	LOS	AM Peak - De Approach	lays LOS	Intersection	LOS	Light	Heavy	Turn	LOS	M Peak - De Approach	lays LOS	Intersection	LOS
			L	3		1	4 35	11 21	3	15 21	25.2 54.4	37.6	27.1 54.4	С					39.3 61.4	62.9	Turn 44.4 61.4	D				
	South	McDonald	R	35		1	97	87	0	87	51.2	67.6	51.3	D	51.4	D			54.6		54.6	D	54.5	D		
			U	0	+	0	0	124	0	0 124	37.2	44.8	37.4	D					31.8		31.8	0				
	East	Finucane	T	1275		34	1,309	1099	24	1,123	33.5	35.1	33.5	С	35.4	D			29.3 68.4	27.1	29.3	С	31.8	с		
1 Finucane /			U	62	-	3	65 0	80	0	80	69.2	84.9	69.9	E			32.8				68.4	E			31.8	с
McDonald			L	206 29	_	1	207 31	88 31	0	88 31	10.1 55.6	35.7 59.6	10.2 55.9	В			32.8	U.	12.7 52.8 52.9		12.7 52.8	B			31.8	L.
	North	McDonald	R	52		1	53	42	0	42	50.4	52.5	50.5	D	22.5	с			52.9		52.9	D	31.0	с		
			L	31		0	32	53	4	0 56	25.9	56.1	26.9	С					25.6	35.1	26.2	с			-	
	West	Finucane	T	1097	-	47	32 1,144 11	53 1345 14	13	56 1,358 14	29.6	30.5	29.6 67.0	С	29.9	с			25.6 29.8 52.2	35.3	26.2 29.8 52.2	C	29.9	с		
			U	0		0	0	0	0	0	63.5	80.2	07.0						32.2		522	U				
	South		T	0	-	0	0	0	0	0										-						
	South		R	0		0	0	0	0	0																
			L	0		0	0	Ö	0	Ö																
	East	Finucane	T	1421	-	38	1,459 107	1289 121	24	1,313 129	2.9 24.2	5.2 19.1	2.9 23.9	A	4.4	A			2.9 27.0	5.0 33.4	2.9 27.4	A D	5.1	A		
2 Finucane / Dawson			U	0 76		0	0 80	0	0	0	6.3	6.6					3.5	А			6.4				3.8	A
Dawson	North	Dawson	T	0		0	0	0	0	Ō			6.3	Α.	12.8	в			6.4			A	14.0	в		
	Notal	Dawson	R	24	_	1	25	17	0	17	32.3	49.0	33.2	D	12.8	в			30.2		30.2	D	14.0	в		
			L	0		0	0	Ő	0	0										_						
	West	Finucane	R	1402	-	49 0	1,451 0	1525	13	1,538	1.9	2.9	2.0	A	2.0	A			2.1	3.1	2.1	A	2.1	A		
L			U	0 514		0	0 514	0 441	0	0 441	32.3		32.3	6					33.5		33.5					
1	South	Delancey	Ť	104		0	104	105	0	105	117.1		117.1	F	47.7	D			33.5 109.7 103.4		109.7	F	50.1	D		
1	03001	Sciancey	R	11	F	0	11	19	0	19	107.2		107.2		-1.1					-7	103.4					
1			L	23		ŏ	23 751	0 751	0	0 778	37.5 40.8		37.5	D					65.3 41.7	100	65.3 41.7	E			1	
1	East	Finucane	R	126		37	130	55	27	56	40.8 84.6	46.1 92.5	41.1 84.9	D	47.3	D				43.0 98.0		D	45.4	D		
3 Finucane / Delancey	L		U	0		0	0 103	28	0	28 81		24.0					60.2	E	77.0 41.0	2015	77.0	E			61.9	E
Deancey	North	Delancey	£ T	91		3	91	80	0	81 162	35.8 91.1	34.8	35.8 91.1	U E	82.3				82.6	I	82.6	U F	75.9			
1	ivonn	Delancey	R	255	+	5	260	218	5	223	97.3	119.8	97.8	F	82.3				83.4	96.5	83.7	F	/5.9			
1		1	L	230		12	241	214	0	0 214	51.2	68.3	52.0	D					50.6	89.6	50.7	D				
1	West	Finucane	R	936 300	-	40	976 300	996 336	12	1,008 336	57.2 108.3	57.5	57.2 108.3		66.5	E			62.2 110.2	71.9	62.3 110.2	E	71.0	E		
			U	0		Ő	0	0	0	0												_				
	South	Wellington	T	64 201	+	13	78	181 228	9	190 230	23.0	17.8 23.3	22.2	B	20.2	с			12.7 12.3	15.6 17.8	12.9	B	14.3	в		
	South	weilington	R	75		2	77	125	1	127	24.2	31.3	24.3	С	20.2	С			20.3	15.4	20.2	С	14.3	в		
			L	64 579		2	66	86	1	87	13.3	15.9	13.4	В					10.7	5.9	10.6	В				
	East	Finucane	T	579	_	17	596 221	636 248	18	654	20.7	21.0 39.0	20.7	C	22.8	с			13.8 16.3	17.0	13.9	B	14.2	в		
4 Finucane /			U	0		0	0		0	0							23.4	с				D			17.7	в
Wellington			L	290 395	-	5	296 405	0 191 244	1	0 192 248	15.8 16.6	20.7 21.7	15.9 16.7	B			23.4	ŭ	9.3 11.0	16.8	9.3 11.0	B			11.7	ŭ
	North	Wellington	R	178		5	182	70	2	71	32.3	30.4	32.2	С	19.6	В			13.8	13.1	13.8	В	10.8	В		
			L	81	+	4	84	137	0	0 138	21.5	32.4	21.9	С					23.4	18.8	23.4	С				
	West	Finucane	T	673 288	_	14	687 314	619 341	5	624	26.5	32.0	26.6 32.7	C	27.9	с			24.8 28.8	23.9	24.8 28.9	C	25.7	с		
			U	5		0	5	19	0	19	32.7 9.9	33.2 46.9	11.4	B					12.0	34.0	12.0	B				
	South		L	296 292	-	0 22	296 314	297 387	0	297 395	44.2 39.1	43.3	44.2 39.4	D	45.5	D			40.0 36.9	42.5	40.0 37.0	D	40.1	D		
	South	Wellington	R	76		3	79	60	0	60	74.5	68.1	74.3	E	45.5	U			60.5		60.5	E	40.1	D		
		-	L	42		2	44	43	3	46	101.0	156.4	103.5	F					59.1 57.9	63.1	59.3	E				
	East	Russell	T	297	_	0	297 103	137 201	0	137 205	103.4 118.4	115.0	103.4 118.2		106.8	F			57.9 59.6		57.9 59.9	E	59.1	E		
5 Wellington /			U	0		0	0	0	0	0							60.0	F							45.6	D
Russell			L	129 538	+	7	135	181	1	182	44.2	45.4	44.3	D			00.0		39.6	32.6	39.6	D			45.0	ŭ
	North	Wellington	R	2		0	2	456 39	0	466 39	41.8 142.2		42.1 142.2	F	42.8	D			39.5 69.4		39.6 69.4	E	41.3	D		
			L	1		0	1	0 51	0	0 51	46.1		46.1	D					52.7		52.7	D				
	West	Russell	T	97 276	-	0	97 276	86 314	0	86 314	56.5 66.6		56.5 66.6	E	63.9	E			52.7 51.9 48.8		51.9 48.8	D	49.8	D		
			U	0		0	0	0	0	0												U				
1			L T	75	+	3	78 418	75 522	2	77 526	3.3 3.2	5.5 5.7	3.3	A					3.0 3.3	4.3 5.5	3.0 3.4	A				
1	South	Wellington	R	15	+	0	15	16	0	16	3.2		3.2	A	3.3	۸			3.2		3.2	A	3.3	^		
1	<u> </u>	1	L	3		0	3	24	0	24	8.5		8.5	A					3.9		3.9	A				
1	East	Freeth	R	0	-	0	0	2	0	2	6.2		6.2	٨	8.4	A			3.1 4.6		3.1 4.6	A	4.0	A		
6 Wellington / Freeth	L		U	1	+			0	0	0							4.8	A	1.9		1.9				3.3	٨
rreeth	North	Wellington	£ T	633		13	646	435	U 6	441	2.1	5.8	2.1	A	5.2				2.4	2.6	2.4	A	2.3			
1			R	58	T	0	58	76	1	77	4.9	4.4	4.9	٨	5.2	^			2.0	2.2	2.0	٨	2.3	~		
1		1	L	69		2	72	105	0	105	6.6	12.5	6.8	A					7.8 9.4		7.8	٨			1	
1	West	Freeth	R	2	+	0	2	4	0	4	5.8	6.4	5.8	A	6.5	A			9.4 4.6	2.3	9.4 4.6	A	6.9	A		
l			U			<i>.</i>		0	0	0									1.0		4.0					
1	e	Dations	T	0 320	+	0	0 331	231	0	231	0.3	0.5	0.3	A	0-				0.1	_	0.1	۸.				
1	South	Delancey	R	122		4	126	135	0	135	1.9	0.5	1.8	Α	0.7	۸			0.4	0.9	0.4	A	0.3	^		
			L	0		0	0 66	0 87	J	0 89	209.1	104.8	208.5	F					24.6	6.5	24.3	С				
7 Delancey / Freeth	East	Freeth	T	0	T	0	0 69	0 68	0	0 69	191.7	160.2	190.4		199.3	F	49.3	E	23.5	9.9	23.2		23.9	с	8.5	A
rideth			U	0		3	0	0	0	0										I		U				
	L		L T	181 380	Ŧ	6	186 387	20 323	0	20 326	46.9 55.1	76.6 49.1	47.8 55.0						5.5 10.5	1.0	5.4 10.4	A				
1	North	Delancey	R	0		0	0	0	0	0					52.6								10.1	В		
			U	0 37	+	0	0 37		0		1.7		1.7	٨					1.5	3.2	1.6	A				
1	South	Delancey	T	37 421		15	37 436 3	27 337 9	0	29 337 9	1.7 0.3 8.4	0.3	1.7 0.3 8.4	A	0.4	A			1.5 0.2 11.4	3.2 0.4	1.6 0.2 11.4	A	0.6	A		
1			U	0		0	0	0	0	0				A								B				
1			L T	5	Ŧ	0	5	7	0	7	179.8		179.8	F					190.1		190.1	F				
l	East	Arthur	R	5		ŏ	5	2	0	2	106.0		106.0	F	141.4				195.6		195.6	F	191.2			
8 Delancey / Arthur			U	0	+	0	0	2	0	2	42.4		42.4				33.8	D	29.3		29.3	D			44.3	E
	North	Delancey	T	431		8	438 14	401	3	404 11	51.6 48.2	53.4	51.7		51.5				33.9 34.6	40.1 24.1	33.9 34.2	D	33.9	D		
1			R	0		0	0	0	0	Ö										I		U				
1			L	16	Ŧ	0	16	26	0	26	258.8		258.8	F					343.7	743.9	346.8	F				
1	West	Site Access	R	9		0	9	49	3	52	367.7		367.7	F	298.6	- F			262.2	298.4	264.0	F	291.9			
	1		U	0		0	0	0	0	0	1															

SCENARIO 1: BACKGROUND TRAFFIC – NO DEVELOPMENT

Scenario 2: Background + development Traffic – No Upgrades

Image Image </th <th></th> <th></th> <th></th> <th></th> <th></th> <th>,</th> <th>Modelle</th> <th>ed Volume</th> <th>s - Development</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>Level of Ser</th> <th>vice with Develo</th> <th>opment Traffic -</th> <th>No Upgrade</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>						,	Modelle	ed Volume	s - Development								Level of Ser	vice with Develo	opment Traffic -	No Upgrade						
<	Intersection	Approach	Description	Tur	n Light	AMP	^v eak		PM Peak	Total	Light	Heavy	Turn	LOS	AM Peak - D Approach	elays LOS					furn I	PM OS /	Approach	lays LOS	Intersection	LOS
10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 <		11		L	3	2		5	13 2	14	18.7	31.2	23.5	C					20.8	63.6	26.2	C				
H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H </td <td></td> <td>South</td> <td>McDonald</td> <td>R</td> <td>113</td> <td>2</td> <td></td> <td>114</td> <td>86 0</td> <td>86</td> <td>28.8</td> <td>92.5</td> <td>20.4 29.6</td> <td>C</td> <td>28.7</td> <td>с</td> <td></td> <td></td> <td>30.4</td> <td>0.0</td> <td>30.4</td> <td>C</td> <td>29.5</td> <td>С</td> <td></td> <td></td>		South	McDonald	R	113	2		114	86 0	86	28.8	92.5	20.4 29.6	C	28.7	с			30.4	0.0	30.4	C	29.5	С		
11111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111 <th< td=""><td></td><td></td><td></td><td>U</td><td></td><td></td><td></td><td></td><td></td><td></td><td>22.4</td><td>43.7</td><td>22.9</td><td>0</td><td></td><td></td><td></td><td></td><td></td><td></td><td>10.3</td><td>B</td><td></td><td></td><td></td><td></td></th<>				U							22.4	43.7	22.9	0							10.3	B				
····································		East	Finucane	т	1373	32	2	1405	1296 25	1321	39.9	44.8	40.0	D	40.5	D			30.9	33.7	31.0	c c	32.3	с		
Image: Protect in transformed protect in tran	1 Finucane /			U							72.9	82.7	12.9								59.1	2			20.7	
1010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010 <td>McDonald</td> <td></td> <td></td> <td>L</td> <td>244</td> <td>2</td> <td></td> <td>246</td> <td>101 0</td> <td>101</td> <td>13.3</td> <td>25.0</td> <td>13.4</td> <td>B</td> <td></td> <td></td> <td>34.0</td> <td>L</td> <td>15.4</td> <td>0.0</td> <td>15.4</td> <td>B</td> <td></td> <td></td> <td>30.7</td> <td>L.</td>	McDonald			L	244	2		246	101 0	101	13.3	25.0	13.4	B			34.0	L	15.4	0.0	15.4	B			30.7	L.
<		North	McDonald	R	50	1			43 0					c	16.8	В			28.5	0.0		c	21.1	с		
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Image: cond bia Image: co		West	Finucane	T		46	5							C	32.8	с						C	30.4	с		
				U	0			0	0 0	0	29.0	00.5	36.0	U					0.0	0.0	32.1	L				
			_	T							27	5.0	27	٨							23	A				
H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H H <		East	Finucane	R	106	3		109	147 9	156				D	4.5	^			29.5	37.9		D	4.9	•		
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····································	2 Finucane / Dawson	North	Dawson	T							22.0	39.2	23.1	c	12.0	в	3.6	A			18.2	0	10.4	В	4.0	A
				U	0	0	_	0	0 0	0		-							0.0	0.0		_				
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			THOUSE	R				0		0														<u> </u>		
<td></td> <td></td> <td></td> <td>L</td> <td>430</td> <td>0</td> <td>1</td> <td>430</td> <td></td> <td></td> <td>81.5</td> <td></td> <td>81.5</td> <td>F</td> <td></td> <td></td> <td></td> <td></td> <td>42.7</td> <td>0.0</td> <td>12.7</td> <td>D</td> <td></td> <td></td> <td></td> <td></td>				L	430	0	1	430			81.5		81.5	F					42.7	0.0	12.7	D				
		South	Delancey	R											99.2	E .						E	59.4	E		
日本 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 <td></td> <td></td> <td></td> <td>U</td> <td></td> <td>0</td> <td></td> <td></td> <td></td> <td></td> <td>20.2</td> <td></td> <td>20.2</td> <td>0</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>24</td> <td><u> </u></td> <td></td> <td></td> <td></td> <td></td>				U		0					20.2		20.2	0							24	<u> </u>				
11111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111 <th< td=""><td></td><td>East</td><td>Finucane</td><td>T</td><td>884</td><td>31</td><td>1</td><td>915</td><td>921 29</td><td>951</td><td>52.6</td><td></td><td>52.8</td><td>D</td><td>67.6</td><td>F</td><td></td><td></td><td>46.7</td><td>47.7</td><td>16.7</td><td>D</td><td>51.4</td><td>D</td><td></td><td></td></th<>		East	Finucane	T	884	31	1	915	921 29	951	52.6		52.8	D	67.6	F			46.7	47.7	16.7	D	51.4	D		
Image: Property image:	3 Finucane /			R						69 25	136.1	150.0	136.4	F								F		-		
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		North	Delancey	R					196 4			127.0		E	49.1	D			40.8	84.2 4		D	36.9	D		
		<u> </u>		L	0 280	10		0 290		0 225	59.2	60.3	59.3	E							34.3	с				
11111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111111 <th< td=""><td></td><td>West</td><td>Finucane</td><td>T</td><td>1001</td><td></td><td>3</td><td>1039</td><td>1104 12</td><td>1116</td><td>64.9</td><td></td><td>65.2</td><td>E</td><td>81.1</td><td>F</td><td></td><td></td><td>82.4</td><td>85.6 1</td><td>32.4</td><td>F</td><td>89.2</td><td>F</td><td></td><td></td></th<>		West	Finucane	T	1001		3	1039	1104 12	1116	64.9		65.2	E	81.1	F			82.4	85.6 1	32.4	F	89.2	F		
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Patenty Free		South	Delancey	R	212	3		215	229 4	233				A	2.3	۸			0.7	1.8		A	0.6	A		
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9 Finance / Bendagment Access / Wet Finance R 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		L BUT	1 HILLING	R	134	0		134	125 0 0 0	125	37.1		37.1	D	0.0				61.8 0.0	0.0 0	51.8	E	11.2	0		
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West Finuaire R 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <th< td=""><td></td><td><u> </u></td><td></td><td>U</td><td>0</td><td></td><td>Ŧ</td><td>0</td><td>0 0</td><td>0</td><td></td><td></td><td></td><td>R</td><td></td><td></td><td></td><td></td><td>0.0</td><td>0.0</td><td></td><td>-</td><td></td><td></td><td></td><td></td></th<>		<u> </u>		U	0		Ŧ	0	0 0	0				R					0.0	0.0		-				
		West	Finucane	T	1503	48	3	1551	1555 12	1567	22.1	53.3	23.1	С	21.4	с			37.7	92.5	38.1	D	36.3	D		
				R											1							_				

Scenario 3: Background + development Traffic – Upgrade 1

				м	odelled	Volumes - w	ith Develop	ment traffic								Level of Se	ervice with Develo	opment Traf	fic - Upgrad	e1					
had a second sec	A	a	Turn Liat		AM Peak			PM Peak	Total	Pake			LOS	AM Peak - D	elays	Intersection	LOS	Light		Turn	F	M Peak - De	elays	later street as	1.05
Intersection	Approach	Description	L 2	2	Heavy 2	4	12	Heavy 2	14	Light 19.8	Heavy 54.6	Turn 35.2	D	Approach	LOS	Intersection	LUS	22.7	Heavy 46.5	25.7	C	Approach	LUS	Intersection	LOS
	South	McDonald	T 37 B 11	7	0	37	27	0	27	29.3	37.3	29.4 29.6	C	29.7	с			31.9 29.4	0.0	31.9 29.4	C	29.5	С		
			U 0)	0	0	0	0	0	27.5				-				0.0	0.0						
			L 92 T 140	2	2	94 1436	116 1295	0 25	116 1320	23.0 41.0	32.6	23.2	C					18.4 30.7	0.0 35.6	18.4 30.8	B				
	East	Finucane	R 71	1	1	72	90	0	90	75.2	39.5 72.2	41.0 75.2	E	41.5	D			76.5	0.0	76.5	E	32.6	с		
1 Finucane / McDonald			U 0	14	2	246	0	0	0	14.9	3.2	14.8	В			35.5	D	0.0	0.0	15.4	В			30.9	с
	North	McDonald	T 30 R 50	0	2	32 51	34 43	0	34 43	26.9	58.5 75.8	29.1 27.9	С	18.2	в			27.4	0.0	27.4	С	21.1	с		
			U 0)	0	0	0	0	0	27.2			C C					29.6 0.0	0.0	29.6	C.				
			L 31 T 127		47	32 1317	52 1519	4	56 1532	13.4 33.5	36.6	14.1 33.7	B	-				12.8 31.1	41.8	14.7 31.1	B				
	West	Finucane	R 9	>	2	11	14	0	14	28.9	71.4	38.0	D	33.3	С			29.5	0.0	29.5	c	30.5	С		
			U 0		0	0	0	0	0									0.0	0.0						
	East	Finucane	T 154	48	32	1579 115	1487	25	1512	1.8	2.6	1.8	A	3.5	٨			1.8	3.0	1.9	A	4.1	А		
			R 11: U 0		3	0	146 0	8	154 0	27.2	41.7	27.5	D					25.6	31.5	26.0	D				
2 Finucane /			L 83 T 0		0	83	44 0	0	44 0	8.5	6.4	8.4	A					5.8 0.0	0.0	5.8	A				
Dawson	North	Dawson	R 29	9	2	30	19	0	19	20.3	44.5	21.7	С	12.0	В	3.1	A	18.7	0.0	18.7	С	9.7	A	3.2	٨
			U 0		0	0	0	0	0					-				0.0	0.0						
	West	Finucane	T 162	23	50	1673	1712	13	1725	2.0	3.2	2.0	A	2.0	Α			2.0	3.1	2.0	A	2.0	A		
			R 0 U 0		0	0	0	0	0					-				0.0	0.0						
			L 61		0	610	587	0	587	22.2		22.2	С					10.2	0.0	10.2	В				
	South	Delancey	T 16 R 22	2	0	168 22	156 20	0	156 20	135.7 54.6		135.7 54.6	D	47.0	D			56.1 30.3	0.0	56.1 30.3	C	20.1	С		
			U 0 L 26)	0	0 26	0 37	0	0 37	75.2		75.2						0.0 62.1	0.0	62.1					
	East	Finucane	T 73	19	31	770	798	32	830	84.2	99.9	84.9	F	104.1				42.5	48.8	42.7	D	45.8	n		
3 Finucane /	Cun	Thouse	R 14 U 0		4	150 0	71 31	3	74	206.4 151.0	235.2	207.1 151.0	F	104.1				75.4 40.5	65.0 0.0	75.0 40.5	E	45.5			
Delancey			L 70	0	3	73	60	1	61	28.6	75.0	30.5	С			58.0	E	33.1	25.8	33.0	С			35.7	D
	North	Delancey	T 11 R 25		5	116 256	107 146	2	107 148	47.2	89.5	47.2 47.8	D	44.8	D			58.8 55.7	0.0 135.7	58.8 56.6	E	52.8	D		
	<u> </u>		U 0)	0	0	0	0	0				-	-				0.0	0.0	8.9					
	West	Finucane	L 16 T 117	71	12 39	173 1210	175 1186	12	176 1198	10.7 35.7	22.0 36.5	11.5 35.7	D	40.5	D			8.9 29.0	3.3 29.3	29.0	c	33.8	0		
		Narutan Na	R 27	15	0	275	352 0	0	352	79.7		79.7	E	40.5	l l			62.7 0.0	0.0	62.7	E		č		
	1		L 36	6	12	47	64	9	73	26.1	18.0	24.1	С					14.8	22.1	15.7	В				
	South	Wellington	T 15 R 76	6	3	160 80	180 115	2	181 118	15.3 14.9	23.6 42.9	15.5 16.1	B	17.1	в			16.4 20.8	17.6 55.3	16.4 21.5	B	17.9	В		
	<u> </u>		U 0)	0	0 104	0 85	0	0 86	35.8	17.6	35.6	D					0.0	0.0	14.9	р			4	
	East	Finucane	T 61	19	14	633	671	20	690	44.2	39.8	44.1	D	46.1	D			29.7	32.7	29.8	C	30.5	c		
4 Finucane /	Cun	Thouse	R 20	07	4	210	267	2	269	57.1	69.1	57.3	E	40.1	, U			37.4	31.6 0.0	37.3	D	30.5	č		
Wellington			L 27	11	4	275	213	2	215	18.0	25.5	18.1	В			29.1	с	12.5	8.2	12.5	В			24.4	с
	North	Wellington	T 39 R 20	19	12	403 211	259 205	5	263 208	27.8	25.9 34.5	27.7 29.1	C	25.1	с			8.5 27.8	20.7 37.9	8.7 27.9	C	15.7	В		
			U 0)	0	0	0	0	0	8.8	25.9	9.2						0.0	0.0	10.5					
	West	Finucane	T 77		15	786	695	5	700	20.7	23.9	20.7	C	22.1	с			24.1	38.7	24.2	C	26.0	c		
			R 36	8	24	392	400	7	407	29.0	32.4	29.2	С		Ŭ			36.5	38.5	36.5	D	20.0	Ŭ		
			L 38	12	0	382	429	0	429	51.9		51.9	D					58.2	0.0	58.2	E				
	South	Wellington	T 26 R 76	6	17	282	325 57	10	335 57	46.2 47.0	53.6 107.2	46.7 48.8	D	49.6	D			46.5 39.7	46.9	46.5 39.7	D	52.2	D		
			U 0		0	0	0 45	0	0 48	86.4	154.2	87.2						0.0	0.0 90.8	37.3	D				
	East	Russell	T 35	9	0	359	213	0	213	180.1		180.1	F	182.5				70.3	0.0	70.3	E	75.9	r.		
5 Wellington /			R 50 U 0		6	56 0	149 0	6	155 0	260.4	256.4	260.0	F					95.6 0.0	94.8 0.0	95.6	F				
Russell			L 16 T 58	i1	5 33	166 620	196 527	0	196 539	47.3 47.2	49.8 48.7	47.4 47.3	D			80.3		47.1 41.9	43.2 44.3	47.1 42.0	D			54.4	
	North	Wellington	R 50	D	0	50	22	0	22	47.2	48.7	47.3	F	52.3	D			92.0	0.0	92.0	F	44.8	D		
			U 0		0	0	0 30	0	0 30	74.7		74.7						0.0 27.4	0.0	27.4	0				
	West	Russell	T 79	9	0	79	89	0	89	66.0		66.0	Ē	80.6				51.8	0.0	51.8	D	54.5	D		
			R 28)	0	285	317 0	0	317 0	84.9		84.9	F					57.8 0.0	0.0	57.8	E		-		
			L 38 T 44		2	40 451	64 561	1	65 564	3.1 1.6	7.3	3.3 1.7	A					3.3 1.8	7.5	3.4	A				
	South	Wellington	R 16		8	451	19	0	19	1.6	7.0	1.7	A	1.8	A			1.7	8.8 0.0	1.8	A A	2.0	A		
			U 0	1	0	14	0 29	0	0 29	6.5		6.5						0.0	0.0	3.6					
	East	Freeth	T 2	2	ő	2	6	Ő	6	3.1		3.1	Å	6.0				4.4	0.0	4.4	Â	3.6	٨		
6 Wellington /			R 0 U 0		0	0	5	0	5									2.1	0.0	2.1	A				
Freeth			L 3	5	0	3	3	0	3	2.3	5.0	2.3	A			3.9	~	2.3	0.0	2.3	A			3.9	^
	North	Wellington	T 64		10 0	655 49	433 81	6 0	439 82	4.6	5.9	4.6	A	4.6	۸			4.4	6.1 13.2	4.4	A	4.4	A		
	<u> </u>	l	U 0)	2	45	0 82	0	0 83	4.8	4.4	4.8						0.0	0.0 20.2	7.6		-		ł	
	West	Freeth	T 11	1	0	11	16	0	16	5.4		5.4	A	5.8	٨			6.5	0.0	6.5	A	7.1	A		
			R 221 U 0	:U)	9	228	215	4	219 0	6.0	6.2	6.0	A .					6.9 0.0	8.3 0.0	7.0	A				
l	1		L 0		0	0	0	0	0	0 E	0.1	05						0.0	0.0	05					
l	South	Delancey	T 415 R 97	7	12 5	427	328 185	4	328 189	0.5	0.1	0.5	A	0.9	٨			0.5	0.2	0.5	A	0.9	A		
			U 0		0	0 68	0	0	0	501.5	18.9	500.1		-				0.0 28.8	0.0	29.1	P				
7 Delancey /	East	Freeth	T 0)	0	0	0	0	0					397.1		56.8		0.0	0.0		_	26.4	D	6.9	
Freeth	L		R 30 U 0)	2	32 0	46 0	0	47	182.1	102.0	177.6						17.4	243.8 0.0	19.4	С				
	[L 17 T 42	19	7	185	145 240	1	146 241	23.6 60.2	65.7 59.6	25.1 60.2	D					3.6	52.5 0.3	4.0 7.9	A				
	North	Delancey	R 0)	0	0	0	0	0	0U.Z	J9.0	00.2		49.6				0.0	0.0	1.9	A	6.4	A		
			U 0		0	0 87	0 61	0	0 63	2.3		2.3						0.0	0.0	2.1					
	South	Delancey	T 38		16	400	337	1	338	0.2	0.3	0.2	A	0.6	A			0.1	0.4	0.1	A	0.4	А		
			R 4		0	4	9	0	9	3.0		3.0	۸					1.4	0.0	1.4	٨				
			L 3 T 0		0	3	6 0	0	6	74.0		74.0	E F					36.1	0.0	36.1	E				
	East	Arthur	R 4	1	0	4	3	0	3	26.3		26.3	D	46.4	E			32.1	0.0	32.1	D	34.7	D		
8 Delancey / Arthur	-		U 0		0	0	0	0	0	0.9		0.9	A			12.9	В	0.0	0.0	14.1	В			8.5	A
	North	Delancey	T 42	2	8	430	301	2	303	23.9	46.3	24.3	С	26.1	D			16.7	33.6	16.8	С	18.5	с		
1	L		R 72 U 0	2	0	72	63 0	0	63 0	36.8		36.8						26.5 0.0	11.1 0.0	26.4	0				
			L 12 T 0	3	0	123 0	172	3	175	6.2		6.2	Α					5.3	3.4	5.3	Α			T I	
1	West	Site Access	R 0)	0	0	0	0	0					6.2	A			0.0	0.0			5.3	A		
├ ───			U 0		0	0	0	0	0	-	-	-						0.0	0.0		_				
1	East	Finucane	T 148	89	36	1524	1408	34	1442	3.9	6.7	3.9	A	6.7	٨			5.2	9.8	5.3	A	10.1	в		
1			R 14 U 0)	0	141 0	124	0	124 0	36.5		36.5	D					65.2 0.0	0.0	65.2					
9 Finucane /			L 13 T 0	13	0	133	171	0	171	21.4		21.4	С	-				17.1	0.0	17.1	В				
Development access	North	Development	R 18	4	0	184	228	0	228	32.9		32.9	с	28.1	с	11.4	В	25.9	0.0	25.9	C	22.2	С	13.5	В
w.LC33	<u> </u>		U 0		0	0 230	0 189	0	0 189	5.0	+	5.0		-				0.0	0.0	5.7					
	West	Finucane	T 148	89	50	1540	1541	0	1553	13.5	28.4	14.0	B	12.8	в			15.4	39.4	15.6	B	14.5	в		
	1		R 0 U 0		0	0	0	0	0	+	-	-	-	۰				0.0	0.0	_		1			
				_								-													

Scenario 4: Background Traffic with development – Upgrade 2

				-	AMP	lled Volume Neak	1		PM Peak						AM Peak - D	elays		ervice with Devel				Р	M Peak - De	elays		
Intersection	Approach	Description	T	'urn Light . 4	Hea 1	wy To		Light 12	Heavy 2	Total 14	Light 22.4	Heavy 51.8	Turn 28.9	LOS	Approach	LOS	Intersection	LOS	Light 18.7	Heavy 56.1	Turn 23.5	LOS	Approach	LOS	Intersection	LOS
	South	McDonald	T	31	0		1	27 87	0	27 87	29.5 28.0	72.6	29.5 28.7	C	28.9	с			29.6	0.0	29.6 30.1	c	29.3	с		
			U	J 0	0	1 0		0	0	0				L					30.1 0.0	0.0		L.				
	East	Finucane	T	95	33	2 14	32	116 1294 91	25	116 1319	23.4 38.7	59.8 42.8	24.0 38.8	C D	39.5	D			18.7 29.9 70.5	36.5 33.8	18.7 30.0	B C	31.5			
1 Finucane /	East	Filiocalite	R	ε 69 J 0	1	7		91 0	0	91 0	74.5	101.4	74.8	E	39.5	U			70.5	0.0	70.5	E	31.5	L.		
McDonald			L	256	2	25	58	101	0	101	16.3	13.9	16.3	В			34.2	С	17.0	0.0	17.0	В			30.6	с
	North	McDonald	R	29	1	3	5	34 43	0	34 43	30.1 27.3	54.3 64.7	31.7 27.9	C	19.3	В			29.2 30.7	0.0	29.2 30.7	C	22.6	С		
			L	J 0 - 31	0	3	2	0 52	4	0 56	12.8	28.9	13.3	В					0.0	0.0 30.0	14.3	В				
	West	Finucane	T	1296	47	7 13	143 9	1512	13	1525 14	32.7	33.3 65.2	32.7 36.3	C	32.3	с			31.2 29.7	35.2	31.2 29.7	C	30.6	с		
			U	J O	0) (0	0	0	0	20.1	05.2	30.3						0.0	0.0	27.1					
	East	Finucane	T	0	32		86	0 1493	0 26	0 1518	1.8	2.8	1.8	٨	3.7				0.0	0.0	1.8	٨	4.2			
			R	107 J 0	3	11	10 D	148	9	157 0	30.9	46.1	31.3	D	5.7	Î			27.1	24.0	26.9	D		<u> </u>		
2 Finucane /			L	85	0			44	0	44	7.7	13.4	7.7	A					8.8	0.0	8.8	٨				
Dawson	North	Dawson	R	25	2	2	.7	19	0	19	19.6	55.5	22.0	С	11.1	В	3.1	A	18.1	0.0	18.1	С	11.6	В	3.3	^
			L	J 0 . 0	0) (0	0	0	0									0.0	0.0						
	West	Finucane	R	1659	0	1 (1706	13	1719 0	2.0	3.0	2.0	A	2.0	۸			2.1	2.8	2.1	۸	2.1	A		
			L	J 0 601	0			0 615	0	0 615	21.8		21.8	с					0.0 9.5	0.0	9.5	Α				
	South	Delancey	T	148	0		48	126 24	0	126 24	126.8 48.8		126.8 48.8	F	42.8	D			83.9 45.8	0.0	83.9 45.8	P	22.9	с		
			U	J 0	0		D	0	Ō	0									0.0	0.0		U				
	East	Finucane	T	13 693	0) 72	3 23	732	0 32	1 764	99.0 49.0	58.7	99.0 49.4	D	66.1				6.7 25.4	0.0 29.0	6.7 25.6	C	29.9	c		
3 Finucane /	East	Filiocalite	R	106 J 8	3	10	09 B	70 23	2	72 23	155.3 258.4	303.4 110.6	159.1	F F	00.1				72.8 39.2	100.8 0.0	73.6 39.2	E D	27.7	č		
Delancey			L	. 77	4	8	12	66 159	1	67 159	20.0	45.2	21.3 29.5	C			48.6	D	22.7 38.8	63.3 0.0	23.3 38.8	C			32.0	с
	North	Delancey	R	300	5	i 30	05	185	2	188	29.5	67.2	30.1	C	28.6	с			38.2	86.6	38.8	D	36.3	D		
			L	J 0 . 170	14	4 18	83	0 204	0	0 205	15.8	33.5	17.1	В					0.0 9.2	0.0 44.6	9.3	٨			ł	
	West	Finucane	T	1200 284	39	9 12	38	1164 331	12	1175 331	45.5 84.2	47.7	45.6 84.2	D	48.9	D			30.6 71.5	30.8	30.6 71.5	C	36.0	D		
			Ū	J 0 49	0		0	0	0	0	31.2	21.3	29.8	0					0.0	0.0	12.8	P				
	South	Wellington	T	219	4	22	23	220	1	221 121	15.6	25.0 23.3	15.7	B	18.7	в			15.1	10.5	15.1	B	13.8	в		
			U	2 0 J 0	0	4	0	0	2	0			23.3	L.					12.3 0.0	24.2	12.5	в				
	East	Finucane	L T	86	2	4 60	18 05	74 684	0	75 703	25.1 41.8	18.0 43.3	25.0 41.8	C D	45.4	D			11.2 24.4	18.3 29.3	11.3 24.5	B	24.1	c		
4 Finucane /	Lun	11100010	R	224	4	22	28 D	267	3	271	63.3	45.2	62.9	E	40.4	Ŭ			26.8 0.0	17.5	26.6	С	24.1	č		
Wellington			L	307 354	4	31	11	207	2	210	16.5	18.9	16.5	В			31.9	с	10.9	13.9	10.9	B			21.2	с
	North	Wellington	R	110	2	11	66 12	257 102	2	261 104	29.1 26.8	25.5 26.8	28.9 26.8	C	23.7	с			10.8 17.5	11.6 21.9	10.8 17.6	B	12.1	В		
			L	J 0 . 111	0	11	14	0	0	0 154	29.2	19.9	29.0	С					0.0	0.0	10.3	В				
	West	Finucane	T	821	15			692 437	5	697 445	28.8	32.4	28.9 33.0	C	30.1	с			24.3 32.9	30.5 37.6	24.3 32.9	C C	25.6	С		
			U	J 0 . 385	0	1 (D	0 426	0	0 426	53.6		53.6	D					32.9 0.0 63.3	0.0	63.3					
	South	Wellington	T	244	18	3 26	61	327	10	337	47.3	48.8	47.4	D	51.6	D			49.2	59.2	49.4	D	56.0	E		
			U	k 68 J 0	3	1 0		58 0	0	58 0	54.7	105.4		2					40.8 0.0	0.0	40.8	U				
	East	Russell	T	. 36 334	1	3	16 34	44	3	47 199	86.3 162.7	216.4	88.5 162.7	F	166.7				31.0 55.5	70.8	33.5 55.5	E	58.3			
5 Wellington /	East	Russell	R	80 J 0	7	8	16 D	162 0	6	168 0	212.8	244.3	215.2	F	100.7				68.7 0.0	69.3 0.0	68.7	E	30.3			
Russell			L	. 148 586	5	15 3 61		202 530	0	202 542	48.5 40.1	47.7	48.5 40.5	D			77.7	Ε	39.0 38.5	62.9 45.4	39.0 38.7	D			50.6	D
	North	Wellington	R	29 J 0	0	2	9	38	0	38	107.0	47.0	107.0	F	44.5	D			43.5	0.0	43.5	D	39.0	D		
			L	. 31	0		1	50	0	50	96.0		96.0	F					28.3	0.0	28.3	С			•	
	West	Russell	T	82 293	0	8	12 93	82 309	0	82 309	65.3 94.9		65.3 94.9	E	89.0	F			52.0 58.1	0.0	52.0 58.1	D E	53.6	D		
			U	J 0 110	0		D	0 92	0	0 94	3.7	5.2	3.8	٨					0.0	0.0 5.5	3.4	٨				
	South	Wellington	T	428	8	43	36	534 17	3	537 17	1.7	6.4	1.7	A	2.1	A			1.7	5.5	1.7	A	2.0	A		
			U	0 1				0	0	0				^					0.0	0.0						
	East	Freeth	T	12	0	4		29 7	0	29 7	6.0 2.5		6.0 2.5	A	5.1				4.0 3.2	0.0	4.0	A	3.6			
6 Wellington /	Lun	- Totali	R	0 5	0		0	5	0	5						Î			1.7	0.0	1.7	٨	3.0	<u> </u>	2.8	
Freeth			L	. <u>3</u> 629	0	1 3	3 39	3 455	0	3 461	2.7	4.5	2.7	A			3.3	A	0.9	0.0 4.0	0.9	A			2.8	٨
	North	Wellington	R	273	0		3	61	1	61	3.5	7.1	3.5	Å	3.9	۸			3.1	6.7	3.1	٨	2.7	۸		
			L	60	2		2	0	1	0	4.2	4.2	4.2	٨					0.0	0.0 8.0	5.2	٨			t	
	West	Freeth	R	12	0		2 42	17 86	0	17 89	2.8	6.4	2.8 4.3	A	4.2	۸			7.2	0.0 5.5	7.2 5.3	A	5.4	A		
			U	J 0 . 0	0			0	0	0			E-						0.0	0.0						
	South	Delancey	T	362	11	1 37	73 22	305 206	0	305 210	0.5	0.3	0.5	A	0.7	A			0.4	0.3	0.4	A	0.5	A		
			Ū	J 0	0	1 0	D	0	0	0									0.0	0.0			I			
7 Delancey /	East	Freeth	T	106	0	1 0	0	99 0	2	100 0	133.5	54.9	133.3		108.2		22.4	с	15.6 0.0	15.2 0.0	15.6	C	12.6	Р	3.7	
Freeth		. 16001	R	R 84	3	8	0	71	1	72 0	73.7	185.1	77.6		130.2		.4.4		8.3 0.0	12.3	8.4	٨	.2.0		5.1	
			L	92 526	5	9	16	14 370	0	14 373	6.9 14.2	24.3 9.3	7.8	A					4.2	1.1	4.2	A				
	North	Delancey	R	200 J 0	0	1 0	0	0	0	0		1.4			13.1	В			0.0	0.0			4.0	۸		
			L	. 70	0	7	0	55	2	57	2.2		2.2	A					2.0	3.2	2.1	٨				
	South	Delancey	R	350	16	1 4	66 4	336 7	0	337 7	0.1	0.4	0.1	A	0.5	۸			0.1	0.5	0.1	A A	0.4	A		
			U	J 0 7	0			0	0	0 8	31.8		31.8	D					0.0 37.0	0.0	37.0					
	East	Arthur	T	0	0	1 (0	0	0	0	23.3		23.3		28.8	D			0.0	0.0	37.9		37.1	E		
8 Delancey /			U	2 4 J 0	0) (0	1	0	1				C			7.3	Δ	37.9	0.0					5.9	A
Arthur	North	Delancey	L	2 524	0	0 53	34	2 399	0	2 402	9.8 10.7	20.2	9.8 10.9	A B	12.0	в			5.8 8.7	0.0 35.5	5.8 8.9	A	10.3	в		
	indrin	Delancey	R	2 100 J 0	0	10	00	69 0	1	69 0	17.8	-	17.8	С	12.0	0			18.6	37.2		С	10.3	0		
		İ	L	126 0		12		172	3	176 0	5.4	-	5.4	٨	-		1		5.1	7.0	5.1	٨			t l	
	West	Site Access	R	2 0	0	0	D	0	0	0					5.4	۸			0.0	0.0			5.1	۸		
			L	J 0 . 0	0) (0	0 0	0	0									0.0	0.0						
	East	Finucane	T	1479	35	5 15	i14 41	1408 125	34	1442 125	4.3 85.9	8.4	4.4 85.9	A	11.3	в			5.1 67.8	10.7	5.2 67.8	A	10.2	в		
	L		U	JO	0		0	0	0	0									0.0	0.0						
9 Finucane / Development	North	Development	T	132 0	0	0		171 0	0	171 0	21.5	1	21.5	C	28.0	с	13.1	в	17.2	0.0	17.2	5	22.2	с	13.1	в
access			R	R 184	0		84 D	228 0	0	228 0	32.7		32.7	С					25.9	0.0	25.9	С				
			L	230	0	23	30	191 1542	0	191 1554	4.8	28.4	4.8 13.3	AB					5.0 14.6	0.0	5.0 14.7	AB				
	West	Finucane	R	k 0 J 0	0) (D D	0	0	0			-		12.2	В			0.0	0.0			13.7	В		
	1		U		1 0		U U	U	U	0	1	1	1	1	1				0.0	0.0			1			

Scenario 5: Background Traffic with development – Upgrade 3

					AM Peak		ith Develop	PM Peak			1			AM Peak - D	elays			lopment Traffic - Upgrade 3	PM Peak - D	elays		
1010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010101010 <td>Intersection</td> <td>Approach</td> <td>Description</td> <td>L 4</td> <td>1 1</td> <td>5</td> <td>12</td> <td>2</td> <td>14</td> <td>17.8</td> <td>47.1</td> <td>24.3</td> <td>LOS</td> <td>Approach</td> <td>LOS</td> <td>Intersection</td> <td>LOS</td> <td>20.8 58.0 25.6 C</td> <td>Approach</td> <td>LOS</td> <td>Intersection</td> <td>LOS</td>	Intersection	Approach	Description	L 4	1 1	5	12	2	14	17.8	47.1	24.3	LOS	Approach	LOS	Intersection	LOS	20.8 58.0 25.6 C	Approach	LOS	Intersection	LOS
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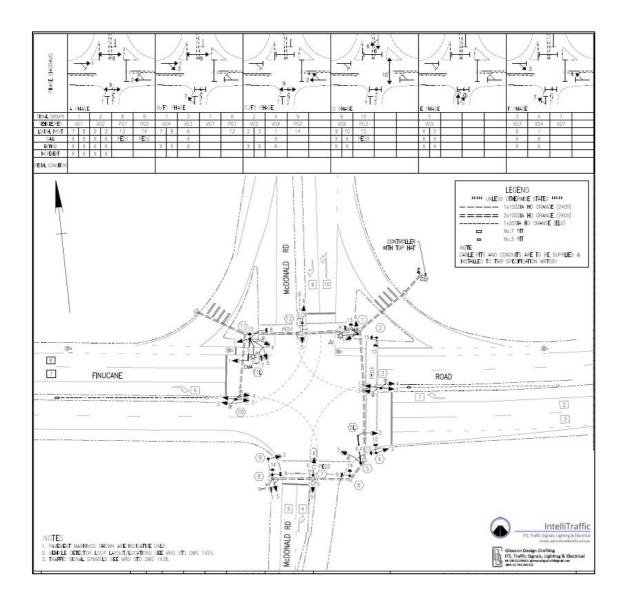
ANNEXURE D – BACKGROUND TRAFFIC NETWORK DELAYS

Background Traffic (Sum of turn volume x delay)			T	otal Delays (s	ec)		
Background frame (sum of turn volume x delay)		AM			PM		Both peaks
Network Scenario	Light	Heavy	Total	Light	Heavy	Total	All traffic
Scenario 1: Base existing network	610316	18747	629062	524603	7970	532573	1161636
Scenario 2: Base existing network with development	780832	25094	805784	607906	11079	618853	1424637
Scenario 3: Development + Upgrade 1 (lengthen slip-lane)	670379	21410	691710	465789	9046	474789	1166498
Scenario 4: Development + Upgrade 2 (slip lane + dual rights)	595133	20335	615284	425934	7899	433747	1049031
Scenario 5: Development + Upgrade 3 (slip lane + dual rights + re-alignment)	595039	20273	615128	425706	7798	433418	1048546

Background Traffic (Sum of turn volume x delay)			То	tal Delays (h	ours		
Backyrounu franc (sun of turn volume'x delay)		AM			PM		Both peaks
Scenario	Light	Heavy	Total	Light	Heavy	Total	All traffic
Scenario 1: Base existing network	170	5	175	146	2	148	323
Scenario 2: Base existing network with development	217	7	224	169	3	172	396
Scenario 3: Development + Upgrade 1 (lengthen slip-lane)	186	6	192	129	3	132	324
Scenario 4: Development + Upgrade 2 (slip lane + dual rights)	165	6	171	118	2	120	291
Scenario 5: Development + Upgrade 3 (slip lane + dual rights + re-alignment)	165	6	171	118	2	120	291

Background traffic % Increase in delay vs Base		AM			PM		Both peaks
Network Scenario	Light	Heavy	Total	Light	Heavy	Total	All traffic
Scenario 2: Base existing network with development	27.9%	33.9%	28.1%	15.9%	39.0%	16.2%	22.6%
Scenario 3: Development + Upgrade 1 (lengthen slip-lane)	9.8%	14.2%	10.0%	-11.2%	13.5%	-10.9%	0.4%
Scenario 4: Development + Upgrade 2 (slip lane + dual rights)	-2.5%	8.5%	-2.2%	-18.8%	-0.9%	-18.6%	-9.7%
Scenario 5: Development + Upgrade 3 (slip lane + dual rights + re-alignment)	-2.5%	8.1%	-2.2%	-18.9%	-2.2%	-18.6%	-9.7%

ANNEXURE E – FINUCANE ROAD / DELANCEY STREET SIGNAL DATA



AM Peak Signal Data

Start: T	:Any>						_																						
Start: T End: T																-													
End: T					_											_													
	hursday	26 May 20	6:00:00																										
No of Cycle	hursday	26 May 20	18:00:00																										
	325																												
No of Disca	0																												
Day T	ime	Cycle No	Cycle Leng	14	1B	10	1D	1E	1F Phase	Stac 1P1WAL	1P2WALK	1P3WALK	Ped Combr	1P1DEMAN	1P2DFM	AT 1P3DEMA	1P1WALK	E 1P2WALK	E1P3WALK	[Plan	CT>60	CT<250				Signal Ph	ase Times		
	26/05/2022 6:58		120		1	10	1			Stag II I WITE	112107121	IT OTHER	None	III IDEIIIII	II LOLINI	a n obeina	II I WITHER		i ii o miten	1336 Plan		TRUE	AM Peak	A	В	C	D	E	F
	26/05/2022 7:00	31	32		6			J 1.	16 AF				None							1336 TMR		TRUE		102.8	#DIV/0!	#DIV/0!	19.6	19.1	18.0
																							Avg Max						
	26/05/2022 7:01	32	169				1						None							1336 TMR		TRUE		125.0	0.0	0.0	36.0	24.0	25.0
	26/05/2022 7:04	33	157				1						None							1336 TMR		TRUE	Min	16.0	0.0	0.0	15.0	13.0	15.0
	26/05/2022 7:06	34	164				1						None							1336 TMR		TRUE							
	26/05/2022 7:09	35	165				1						None							1336 TMR		TRUE							
	26/05/2022 7:12	36	152				1						None							1336 TMR		TRUE							
	26/05/2022 7:14	37	143				1		17 ADF				None							1336 TMR		TRUE							
4	26/05/2022 7:17	38	177	12	!5		1	5 19	18 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 7:20	39	143	10	18		1	9	16 ADF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 7:22	40	178	12	4		1	7 2	16 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 7:25	41	160) 10	16		1	5 20	19 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 7:28	42	159	10	16		1	8 19	16 ADEF				None							1336 TMR	TRUE	TRUE							
	26/05/2022 7:30	43	168		18		1						None							1336 TMR		TRUE							
	26/05/2022 7:33	44	160				1						None							1336 TMR		TRUE							
	26/05/2022 7:36	45	148				1						None							1336 TMR		TRUE							
	26/05/2022 7:38	46	166				1						None							1336 TMR		TRUE							
	26/05/2022 7:41	40	158				1						None							1336 TMR		TRUE							
	26/05/2022 7:41	47	165				1						None	42						1336 TMR		TRUE							
		48	100				3				•			42		10	1A		10			TRUE							
	26/05/2022 7:46									1	J	12	1 3			18	IA		1D	1336 TMR									
	26/05/2022 7:49	50	144		2		1						None							1336 TMR		TRUE							
	26/05/2022 7:52	51	165				1						None							1336 TMR		TRUE							
	26/05/2022 7:54	52	171				3					12				42			1D	1336 TMR		TRUE							
	26/05/2022 7:57	53	152		2		1						None							1336 TMR		TRUE							
4	26/05/2022 8:00	54	155				1						None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:02	55	167	10	15		1	8 20	24 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:05	56	153	5 9	18		1	9 20	16 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:08	57	159) 10	15		1	6 22	16 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:10	58	163	10	16		1	9 20	18 ADEF		4		2		13	6		1F		1336 TMR	TRUE	TRUE							
4	26/05/2022 8:13	59	171	10	13		3	6 13	19 ADEF		4	12	2 3			96		1A	1D	1336 TMR	TRUE	TRUE							
	26/05/2022 8:16	60	143		2		1						None							1336 TMR		TRUE							
	26/05/2022 8:18	61	177				3					12				10			1D	1336 TMR		TRUE							-
	26/05/2022 8:21	62	150		2		1						None			10				1336 TMR		TRUE							-
	26/05/2022 8:24	63	150				1						None							1336 TMR		TRUE							
	26/05/2022 8:26	64	164				1						None							1336 TMR		TRUE							-
			153		19		1						None							1336 TMR		TRUE							+
	26/05/2022 8:29 26/05/2022 8:32	65 66	153				1						None							1336 TMR 1336 TMR		TRUE							+
							1															TRUE							+
	26/05/2022 8:34	67	163										None			11			10	1336 TMR									
	26/05/2022 8:37	68	167				3					12				101			1D	1336 TMR		TRUE							
	26/05/2022 8:40	69	153		5		1						None							1336 TMR		TRUE							
	26/05/2022 8:42	70	158				1						None							1336 TMR		TRUE							
	26/05/2022 8:45	71	144					24					None							1336 TMR		TRUE							
	26/05/2022 8:47	72	174				1						None							1336 TMR		TRUE							
4	26/05/2022 8:50		162				1						None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:53	74	160) 10	14		1	8 20	18 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:56	75	159	9 10	14		1	9 19	17 ADEF				None							1336 TMR	TRUE	TRUE							
4	26/05/2022 8:58	76	160) 10	15		1	5 15	25 ADEF				None							1336 TMR	TRUE	TRUE							

PM Peak Signal Data

	M1336/CTLR											_			_												
												-															
	<any></any>																				-						
	Thursday		6:00:00																								
		26 May 20	18:00:00																								
No of Cycle	325																										
No of Disca	0																										
4	26/05/2022 14:57	242	133	74		19	19	21 ADEF			None						13	336 Plan(TRUE	TRUE				Signal Ph	ase Times		
4	26/05/2022 14:59	243	138	81		16	23	18 ADEF			None						13	336 Plan(TRUE	TRUE	PM Peak	A	В	С	D	E	F
4	26/05/2022 15:01		137	83		18	16	20 ADEF			None							1336 Plan(TRUE	TRUE	Ava	80.0	#DIV/0!	#DIV/0!	18.9	17.2	18.3
4	26/05/2022 15:04			86		19	20	15 ADEF			None								TRUE	TRUE	Max	96.0	0.0	0.0	36.0	25.0	25.0
4	26/05/2022 15:06		145	86		19	15	25 ADEF			None								TRUE	TRUE	Min	25.0	0.0	0.0	14.0	13.0	14.0
4	26/05/2022 15:08		145	81		20	19	25 ADEF			None								TRUE	TRUE		20.0	0.0	0.0	14.0	10.0	14.0
	26/05/2022 15:00		133	76		19	20	18 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:13		137	83		17	17	20 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:15		148	86		18	19	25 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:18		132	78		15	18	21 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:20		145	86		18	17	24 ADEF			None						13	1336 Plan('	TRUE	TRUE							
4	26/05/2022 15:22	253	141	81		19	21	20 ADEF			None						13	1336 Plan('	TRUE	TRUE							
4	26/05/2022 15:25	254	146	80		36	14	16 ADEF		11		3		21		10) 13	336 Plan('	TRUE	TRUE							
4	26/05/2022 15:27	255	136	74		15	25	22 ADEF			None						13	336 Plan(TRUE	TRUE							
4	26/05/2022 15:29		138	78		20	19	21 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:32		146	80		36	14	16 ADEF		10		3		28		10			TRUE	TRUE	i						
4	26/05/2022 15:34		127	74		17	16	20 ADEF	5			2	34	29	1F				TRUE	TRUE							
	26/05/2022 15:34		153	87		36	14	16 ADEF			2 3	-	34	87	14	10			TRUE	TRUE		-					
									-	5 IU				0/	IA							-					
4	26/05/2022 15:39		122	74		15	16	17 ADEF			None	~							TRUE	TRUE	-						
4	26/05/2022 15:41		158	92		36	14	16 ADEF		10		3		52		10			TRUE	TRUE							
4	26/05/2022 15:44		130	74		19	20	17 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:46		143	84		19	20	20 ADEF			None								TRUE	TRUE							
4	26/05/2022 15:48	264	136	81		16	23	16 ADEF			None						13	1336 Plan('	TRUE	TRUE							
4	26/05/2022 15:50	265	140	85		19	20	16 ADEF			None						13	336 Plan(TRUE	TRUE							
4	26/05/2022 15:53	266	146	85		18	22	21 ADEF			None						13	336 Plan(TRUE	TRUE							
4	26/05/2022 15:55	267	134	79		18	22	15 ADEF			None						13	336 Plan(TRUE	TRUE	i i						
4	26/05/2022 15:57		136	85		19	16	16 ADEF			None								TRUE	TRUE	i i						
	26/05/2022 16:00		72	25		16	15	16 ADEF			None								TRUE	TRUE	i						
	26/05/2022 16:00			83		15	17	15 ADEF			None								TRUE	TRUE							
			130	83			17	17 ADEF												TRUE							
4	26/05/2022 16:03					16					None								TRUE								
4	26/05/2022 16:05		129	81		14	16	18 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:07			82		16	16	18 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:10		127	79		17	16	15 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:12	275	117	82			18	17 AEF			None						13	1336 Plan('	TRUE	TRUE							
4	26/05/2022 16:14	276	144	95		17	15	17 ADEF			None						13	336 Plan(TRUE	TRUE							
4	26/05/2022 16:16		130	81		14	19	16 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:18		131	81		17	15	18 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:20		131	80		15	17	19 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:23		130	79		16	16	19 ADEF			None								TRUE	TRUE	i						
4	26/05/2022 16:25		130	79		15	17	19 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:25		130	79		16	17	19 ADEF			None								TRUE	TRUE		-					
4	26/05/2022 16:27		129	78		16	17	18 ADEF			None								TRUE	TRUE							
4			129	78		16		18 ADEF 18 ADF											TRUE	TRUE		-					
4	26/05/2022 16:31										None																
4	26/05/2022 16:33		147	96		15	18	18 ADEF			None								TRUE	TRUE		-					
4	26/05/2022 16:36		128	79		15	18	16 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:38		144	81		36	13	14 ADEF		10		3	109	2		10			TRUE	TRUE							
4	26/05/2022 16:40		116	67		17	16	16 ADEF	ç)		2			1A				TRUE	TRUE							
4	26/05/2022 16:42	289	132	81		15	17	19 ADEF			None						13	336 Plan('	TRUE	TRUE							
4	26/05/2022 16:44	290	130	79		16	16	19 ADEF			None						13	336 Plan(TRUE	TRUE							
4	26/05/2022 16:46		130	79		16	16	19 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:49			79		16	16	19 ADEF			None								TRUE	TRUE	i						
4	26/05/2022 16:51			79		18	16				None								TRUE	TRUE		-					
4								18 ADEF													-	-					
4	26/05/2022 16:53			79		17	14	19 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:55		131	79		14	18	20 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:57		129	78		16	16	19 ADEF			None								TRUE	TRUE							
4	26/05/2022 16:59		143	79		36	13	15 ADEF		10		3		66		10			TRUE	TRUE							
	26/05/2022 17:02	298	138	66	46		26	AEC			None						41	336 Plan(TOULE	TRUE							



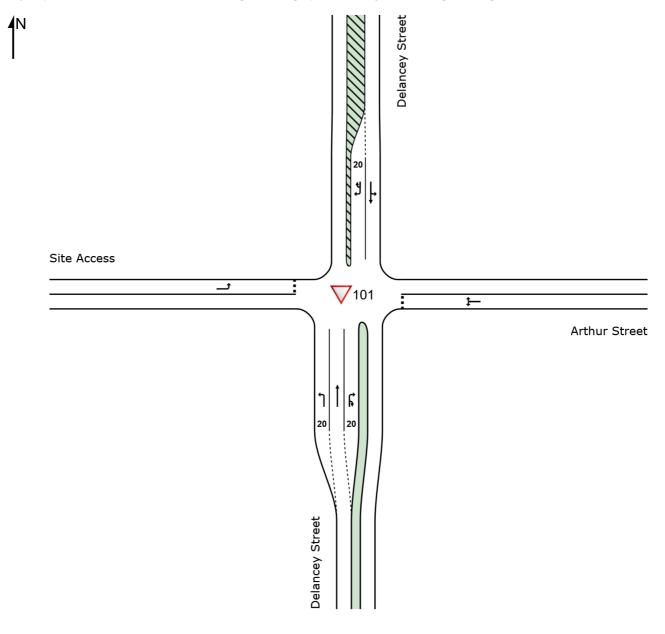
APPENDIX E- SIDRA OUTPUTS

SITE LAYOUT

V Site: 101 [Delancey Street Site Access W/D 2024 AM (Site Folder: Access SIDRAS)]

New Site Site Category: (None) Give-Way (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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V Site: 101 [Delancey Street Site Access W/D 2024 AM (Site Folder: Access SIDRAS)]

New Site Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total	MES HV]	لDEM FLO Total]	WS HV]	Deg. Satn	Delay	Level of Service	QUE [Veh.	ACK OF EUE Dist]	Prop. E Que	ffective Stop Rate	Aver. No. S Cycles	Aver. Speed
South	a: Dolo	veh/h Incey Stre	%	veh/h	%	v/c	sec	-	veh	m	-		-	km/h
		,		~~	~ ~									
1	L2	79	0.0	83	0.0	0.045	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	53.6
2	T1	329	4.0	346	4.0	0.182	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
3	R2	6	0.0	6	0.0	0.008	7.7	LOS A	0.0	0.2	0.54	0.65	0.54	51.0
3u	U	1	0.0	1	0.0	0.008	11.7	LOS B	0.0	0.2	0.54	0.65	0.54	51.1
Appr	oach	415	3.2	437	3.2	0.182	1.2	NA	0.0	0.2	0.01	0.12	0.01	58.4
East:	Arthu	r Street												
4	L2	7	0.0	7	0.0	0.024	8.6	LOS A	0.1	0.5	0.61	0.76	0.61	49.4
6	R2	3	0.0	3	0.0	0.024	18.0	LOS C	0.1	0.5	0.61	0.76	0.61	49.2
Appr	oach	10	0.0	11	0.0	0.024	11.4	LOS B	0.1	0.5	0.61	0.76	0.61	49.3
North	n: Dela	ncey Stre	et											
7	L2	2	0.0	2	0.0	0.301	5.6	LOS A	0.0	0.0	0.00	0.00	0.00	58.2
8	T1	545	2.0	574	2.0	0.301	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.8
9	R2	95	0.0	100	0.0	0.111	7.6	LOS A	0.4	3.1	0.48	0.70	0.48	51.5
9u	U	1	0.0	1	0.0	0.111	9.3	LOS A	0.4	3.1	0.48	0.70	0.48	51.6
Appr	oach	643	1.7	677	1.7	0.301	1.2	NA	0.4	3.1	0.07	0.11	0.07	58.4
West	: Site A	Access												
10	L2	126	0.0	133	0.0	0.143	7.3	LOS A	0.5	3.7	0.42	0.67	0.42	52.3
Appr	oach	126	0.0	133	0.0	0.143	7.3	LOS A	0.5	3.7	0.42	0.67	0.42	52.3
All Vehic	les	1194	2.0	1257	2.0	0.301	2.0	NA	0.5	3.7	0.09	0.18	0.09	57.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Delancey Street Site Access W/D 2024 PM (Site Folder: Access SIDRAS)]

New Site Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM/ FLO ^V [Total veh/h		Deg. Satn v/c		Level of Service		ACK OF EUE Dist] m	Prop. E Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
Sout	h: Dela	ancey Stre												
1	L2	59	4.0	62	4.0	0.034	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	53.4
2	T1	322	0.0	339	0.0	0.174	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
3	R2	9	0.0	9	0.0	0.010	6.9	LOS A	0.0	0.3	0.46	0.61	0.46	51.8
3u	U	1	0.0	1	0.0	0.010	9.8	LOS A	0.0	0.3	0.46	0.61	0.46	51.9
Appr	oach	391	0.6	412	0.6	0.174	1.1	NA	0.0	0.3	0.01	0.10	0.01	58.6
East	Arthu	r Street												
4	L2	6	0.0	6	0.0	0.014	7.5	LOS A	0.0	0.3	0.50	0.67	0.50	50.9
6	R2	2	0.0	2	0.0	0.014	14.6	LOS B	0.0	0.3	0.50	0.67	0.50	50.6
Appr	oach	8	0.0	8	0.0	0.014	9.3	LOS A	0.0	0.3	0.50	0.67	0.50	50.8
North	n: Dela	ncey Stre	et											
7	L2	3	1.0	3	1.0	0.225	5.6	LOS A	0.0	0.0	0.00	0.00	0.00	58.2
8	T1	408	1.0	429	1.0	0.225	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.8
9	R2	69	0.0	73	0.0	0.078	7.3	LOS A	0.3	2.1	0.45	0.67	0.45	51.7
9u	U	1	0.0	1	0.0	0.078	9.1	LOS A	0.3	2.1	0.45	0.67	0.45	51.8
Appr	oach	481	0.9	506	0.9	0.225	1.2	NA	0.3	2.1	0.07	0.10	0.07	58.5
West	: Site A	Access												
10	L2	176	2.0	185	2.0	0.199	7.4	LOS A	0.8	5.5	0.43	0.68	0.43	52.2
Appr	oach	176	2.0	185	2.0	0.199	7.4	LOS A	0.8	5.5	0.43	0.68	0.43	52.2
All Vehic	cles	1056	0.9	1112	0.9	0.225	2.2	NA	0.8	5.5	0.11	0.20	0.11	57.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Delancey Street Site Access W/D 2034 AM (Site Folder: Access SIDRAS)]

New Site Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU	IMES	DEM/ FLO	WS	Deg. Satn		Level of Service	QUE	ACK OF EUE	Prop. E Que	ffective Stop	Aver. No.	Aver. Speed
		[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
Sout	h: Dela	ncey Stre	eet											
1	L2	79	0.0	83	0.0	0.045	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	53.6
2	T1	440	4.0	463	4.0	0.244	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
3	R2	7	0.0	7	0.0	0.011	8.2	LOS A	0.0	0.3	0.57	0.68	0.57	50.7
3u	U	1	0.0	1	0.0	0.011	12.7	LOS B	0.0	0.3	0.57	0.68	0.57	50.8
Appr	oach	527	3.3	555	3.3	0.244	1.0	NA	0.0	0.3	0.01	0.10	0.01	58.7
East:	Arthu	r Street												
4	L2	8	0.0	8	0.0	0.037	9.2	LOS A	0.1	0.8	0.69	0.82	0.69	47.7
6	R2	4	0.0	4	0.0	0.037	23.6	LOS C	0.1	0.8	0.69	0.82	0.69	47.5
Appr	oach	12	0.0	13	0.0	0.037	14.0	LOS B	0.1	0.8	0.69	0.82	0.69	47.6
North	n: Dela	ncey Stre	et											
7	L2	3	0.0	3	0.0	0.335	5.6	LOS A	0.0	0.0	0.00	0.00	0.00	58.2
8	T1	605	2.0	637	2.0	0.335	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.8
9	R2	95	0.0	100	0.0	0.129	8.5	LOS A	0.5	3.5	0.54	0.77	0.54	50.9
9u	U	1	0.0	1	0.0	0.129	10.7	LOS B	0.5	3.5	0.54	0.77	0.54	50.9
Appr	oach	704	1.7	741	1.7	0.335	1.3	NA	0.5	3.5	0.07	0.11	0.07	58.4
West	: Site A	Access												
10	L2	126	0.0	133	0.0	0.166	8.2	LOS A	0.6	4.2	0.49	0.74	0.49	51.6
Appr	oach	126	0.0	133	0.0	0.166	8.2	LOS A	0.6	4.2	0.49	0.74	0.49	51.6
All Vehic	cles	1369	2.2	1441	2.2	0.335	1.9	NA	0.6	4.2	0.09	0.17	0.09	57.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Delancey Street Site Access W/D 2034 PM (Site Folder: Access SIDRAS)]

New Site Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total	MES HV]	DEMA FLO [Total	WS HV]	Deg. Satn	Delay	Level of Service	QUI [Veh.	ACK OF EUE Dist]	Prop. E Que	ffective Stop Rate	Aver. No. Cycles	Aver. Speed
Sout	n: Dela	veh/h incey Stre	% eet	veh/h	%	v/c	sec	_	veh	m	_	_	_	km/h
1	L2	59	4.0	62	4.0	0.034	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	53.4
2	T1	359	0.0	378	0.0	0.194	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
3	R2	10	0.0	11	0.0	0.011	7.1	LOS A	0.0	0.3	0.48	0.62	0.48	51.7
3u	U	1	0.0	1	0.0	0.011	10.2	LOS B	0.0	0.3	0.48	0.62	0.48	51.7
Appr	oach	429	0.6	452	0.6	0.194	1.0	NA	0.0	0.3	0.01	0.10	0.01	58.7
East:	Arthu	r Street												
4	L2	7	0.0	7	0.0	0.017	7.7	LOS A	0.1	0.4	0.52	0.68	0.52	50.6
6	R2	2	0.0	2	0.0	0.017	16.2	LOS C	0.1	0.4	0.52	0.68	0.52	50.4
Appr	oach	9	0.0	9	0.0	0.017	9.6	LOS A	0.1	0.4	0.52	0.68	0.52	50.6
North	n: Dela	ncey Stre	et											
7	L2	4	1.0	4	1.0	0.245	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	58.2
8	T1	444	1.0	467	1.0	0.245	0.1	LOS A	0.0	0.0	0.00	0.01	0.00	59.8
9	R2	69	0.0	73	0.0	0.082	7.6	LOS A	0.3	2.2	0.47	0.69	0.47	51.5
9u	U	1	0.0	1	0.0	0.082	9.5	LOS A	0.3	2.2	0.47	0.69	0.47	51.6
Appr	oach	518	0.9	545	0.9	0.245	1.1	NA	0.3	2.2	0.06	0.10	0.06	58.5
West	: Site A	Access												
10	L2	176	2.0	185	2.0	0.208	7.6	LOS A	0.8	5.7	0.46	0.71	0.46	52.0
Appr	oach	176	2.0	185	2.0	0.208	7.6	LOS A	0.8	5.7	0.46	0.71	0.46	52.0
All Vehic	les	1132	0.9	1192	0.9	0.245	2.2	NA	0.8	5.7	0.11	0.20	0.11	57.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

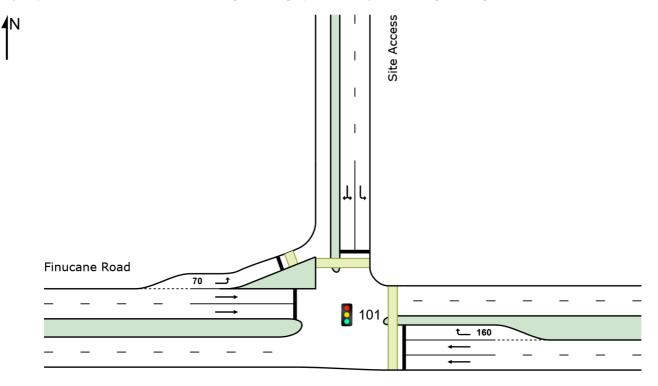
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SITE LAYOUT

Site: 101 [Finucane Road Site Access W/D 2024 AM (Site Folder: Access SIDRAS)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Finucane Road

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Site: 101 [Finucane Road Site Access W/D 2024 AM (Site

Folder: Access SIDRAS)]

New Site

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

Vehi	cle M	ovemen	t Perfoi	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM, FLO [Total veh/h		Deg. Satn		Level of Service	95% BA QUE [Veh. veh	EUE Dist]	Prop. E Que	ffective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
East:	Finuc	ane Road		ven/n	70	v/c	Sec	_	ven	m	_	_	_	KIII/11
5	T1	1548	2.0	1629	2.0	0.595	6.9	LOS A	17.7	126.0	0.54	0.50	0.54	53.9
6 Appro	R2 bach	142 1690	0.0 1.8	149 1779	0.0 1.8	* 0.604 0.604	47.1 10.3	LOS D LOS B	6.5 17.7	45.4 126.0	0.99 0.58	0.81 0.52	1.01 0.58	33.3 51.2
North	: Site	Access												
7	L2	133	0.0	140	0.0	0.212	27.3	LOS C	4.3	30.0	0.74	0.75	0.74	40.7
9	R2	142	0.0	149	0.0	* 0.517	44.5	LOS D	6.2	43.7	0.97	0.80	0.97	34.2
Appro	oach	275	0.0	289	0.0	0.517	36.2	LOS D	6.2	43.7	0.85	0.78	0.85	37.0
West	: Finuc	ane Roa	d											
10 11	L2 T1	230 1124	0.0 4.0	242 1183	0.0 4.0	0.178 * 0.620	9.5 16.7	LOS A LOS B	3.2 18.9	22.4 136.5	0.33 0.76	0.65 0.68	0.33 0.76	51.4 47.0
Appro	bach	1354	3.3	1425	3.3	0.620	15.5	LOS B	18.9	136.5	0.69	0.68	0.69	47.7
All Vehic	les	3319	2.3	3494	2.3	0.620	14.6	LOS B	18.9	136.5	0.65	0.61	0.65	48.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian M	Noveme	ent Peri	ormano	e							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of <i>J</i> Service	AVERAGE QUE [Ped		Prop. Ef Que	fective Stop Rate	Travel Time	Travel Dist. S	Aver. Speed
	ped/h	ped/h	sec		ped	m		naic	sec	m	m/sec
East: Finucane	e Road										
P2 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	208.9	220.5	1.06
North: Site Acc	cess										
P3 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	203.8	213.9	1.05
West: Finucan	e Road										
P4B ^{Slip/} Bypass	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	196.4	204.3	1.04
All Pedestrians	150	158	39.3	LOS D	0.1	0.1	0.94	0.94	203.1	212.9	1.05

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. SIDRA INTERSECTION 9.0 | Copyright © 2000-2020 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: LAMBERT AND REHBEIN (SEQ) PTY LTD | Licence: PLUS / 1PC | Processed: Monday, 11 July 2022 12:02:53 PM Project: F:\Jobs\B19500\B19590\Design\Traffic\B19590 SIDRAS - Network.sip9

PHASING SUMMARY

Site: 101 [Finucane Road Site Access W/D 2024 AM (Site

Folder: Access SIDRAS)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

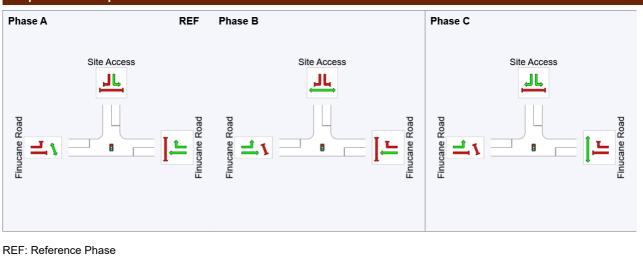
Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Leading Right Turn Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Summary

Phase	Α	В	С
Phase Change Time (sec)	0	18	70
Green Time (sec)	12	46	14
Phase Time (sec)	18	52	20
Phase Split	20%	58%	22%

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

Output Phase Sequence



VAR: Variable Phase



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Site: 101 [Finucane Road Site Access W/D 2024 PM (Site

Folder: Access SIDRAS)]

New Site

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

Vehi	cle M	ovement	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM, FLO [Total veh/h		Deg. Satn v/c		Level of Service	95% BA QUE [Veh. veh		Prop. E Que	ffective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
East:	Finuc	ane Road	l											
5 6 Appro	T1 R2 bach	1441 124 1565	2.0 0.0 1.8	1517 131 1647	2.0 0.0 1.8	0.601 * 0.633 0.633	9.4 49.4 12.6	LOS A LOS D LOS B	18.7 5.8 18.7	133.3 40.9 133.3	0.62 1.00 0.65	0.56 0.82 0.58	0.62 1.06 0.65	52.0 32.6 49.6
North	: Site	Access												
7 9 Appro	L2 R2 bach	171 228 399	0.0 0.0 0.0	180 240 420	0.0 0.0 0.0	0.249 *0.612 0.612	25.5 41.1 34.4	LOS C LOS D LOS C	5.3 9.8 9.8	37.4 68.6 68.6	0.72 0.96 0.85	0.76 0.82 0.79	0.72 0.96 0.85	41.5 35.3 37.7
West	: Finuc	cane Road	d											
10 11	L2 T1	190 1083	0.0 1.0	200 1140	0.0 1.0	0.143 * 0.623	8.8 18.7	LOS A LOS B	2.4 18.8	16.5 132.6	0.29 0.80	0.64 0.71	0.29 0.80	51.9 45.9
Appro	oach	1273	0.9	1340	0.9	0.623	17.2	LOS B	18.8	132.6	0.72	0.70	0.72	46.7
All Vehic	les	3237	1.2	3407	1.2	0.633	17.1	LOS B	18.8	133.3	0.70	0.65	0.70	46.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian M	loveme	ent Peri	formand	:e							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of a Service	AVERAGE QUE [Ped		Prop. Ef Que	fective Stop Rate	Travel Time	Travel Dist. S	Aver. Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
East: Finucane	e Road										
P2 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	208.9	220.5	1.06
North: Site Acc	cess										
P3 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	203.8	213.9	1.05
West: Finucan	e Road										
P4B ^{Slip/} Bypass	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	196.4	204.3	1.04
All Pedestrians	150	158	39.3	LOS D	0.1	0.1	0.94	0.94	203.1	212.9	1.05

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. SIDRA INTERSECTION 9.0 | Copyright © 2000-2020 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: LAMBERT AND REHBEIN (SEQ) PTY LTD | Licence: PLUS / 1PC | Processed: Monday, 11 July 2022 12:02:54 PM Project: F:\Jobs\B19500\B19590\Design\Traffic\B19590 SIDRAS - Network.sip9

PHASING SUMMARY

Site: 101 [Finucane Road Site Access W/D 2024 PM (Site

Folder: Access SIDRAS)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

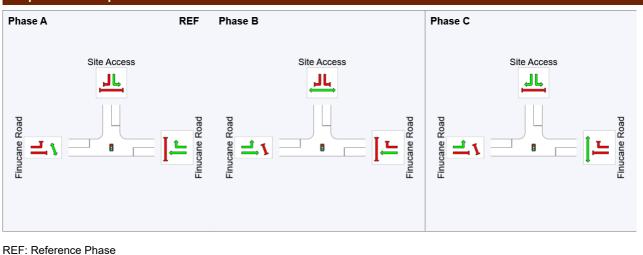
Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Leading Right Turn Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Summary

Phase	Α	В	С
Phase Change Time (sec)	0	16	65
Green Time (sec)	10	43	19
Phase Time (sec)	16	49	25
Phase Split	18%	54%	28%

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

Output Phase Sequence



VAR: Variable Phase



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Site: 101 [Finucane Road Site Access W/D 2034 AM (Site

Folder: Access SIDRAS)]

New Site

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

Vehi	cle M	ovement	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM, FLO [Total veh/h		Deg. Satn v/c		Level of Service	95% BA QUE [Veh. veh		Prop. E Que	ffective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
East:	Finuc	ane Road	l											
5 6 Appro	T1 R2 bach	1656 142 1798	2.0 0.0 1.8	1743 149 1893	2.0 0.0 1.8	0.637 * 0.659 0.659	7.3 48.9 10.6	LOS A LOS D LOS B	20.0 6.7 20.0	142.1 46.7 142.1	0.57 1.00 0.61	0.53 0.83 0.55	0.57 1.07 0.61	53.6 32.8 51.0
North	: Site	Access												
7 9 Appro	L2 R2 bach	133 184 317	0.0 0.0 0.0	140 194 334	0.0 0.0 0.0	0.219 * 0.670 0.670	28.1 46.4 38.7	LOS C LOS D LOS D	4.4 8.5 8.5	30.5 59.2 59.2	0.75 0.99 0.89	0.75 0.84 0.80	0.75 1.05 0.93	40.3 33.6 36.1
West	: Finuc	cane Road	d											
10 11	L2 T1	230 1287	0.0 4.0	242 1355	0.0 4.0	0.175 * 0.713	9.2 17.1	LOS A LOS B	3.1 23.6	21.4 170.8	0.31 0.80	0.65 0.72	0.31 0.80	51.6 46.8
Appro	bach	1517	3.4	1597	3.4	0.713	15.9	LOS B	23.6	170.8	0.73	0.71	0.73	47.5
All Vehic	les	3632	2.3	3823	2.3	0.713	15.3	LOS B	23.6	170.8	0.68	0.64	0.69	47.8

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance											
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of A Service	AVERAGE BACK OF QUEUE [Ped Dist]		Prop. Effective Que Stop Rate		Travel Time	Travel Dist. S	Aver. Speed
	ped/h	ped/h	sec		ped	m		Trate	sec	m	m/sec
East: Finucane	e Road										
P2 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	208.9	220.5	1.06
North: Site Acc	cess										
P3 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	203.8	213.9	1.05
West: Finucan	e Road										
P4B ^{Slip/} Bypass	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	196.4	204.3	1.04
All Pedestrians	150	158	39.3	LOS D	0.1	0.1	0.94	0.94	203.1	212.9	1.05

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. SIDRA INTERSECTION 9.0 | Copyright © 2000-2020 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: LAMBERT AND REHBEIN (SEQ) PTY LTD | Licence: PLUS / 1PC | Processed: Monday, 11 July 2022 12:02:55 PM Project: F:\Jobs\B19500\B19590\Design\Traffic\B19590 SIDRAS - Network.sip9

PHASING SUMMARY

Site: 101 [Finucane Road Site Access W/D 2034 AM (Site

Folder: Access SIDRAS)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

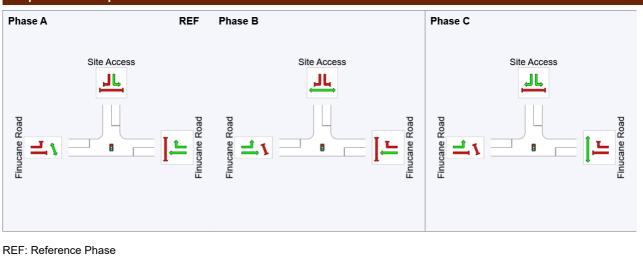
Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Leading Right Turn Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Summary

Phase	Α	В	С
Phase Change Time (sec)	0	17	70
Green Time (sec)	11	47	14
Phase Time (sec)	17	53	20
Phase Split	19%	59%	22%

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

Output Phase Sequence



VAR: Variable Phase



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Site: 101 [Finucane Road Site Access W/D 2034 PM (Site

Folder: Access SIDRAS)]

New Site

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

Vehi	cle M	ovemen	t Perfo	rmance										
	Turn	INP VOLU [Total veh/h	UT	DEM, FLO [Total veh/h		Deg. Satn v/c		Level of Service	95% BA QUI [Veh. veh	ACK OF EUE Dist] m	Prop. E Que	ffective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
East:	East: Finucane Road													
5 6 Appro	T1 R2 bach	1589 124 1713	2.0 0.0 1.9	1673 131 1803	2.0 0.0 1.9	0.641 *0.633 0.641	8.8 49.4 11.7	LOS A LOS D LOS B	20.7 5.8 20.7	147.3 40.9 147.3	0.62 1.00 0.65	0.57 0.82 0.59	0.62 1.06 0.65	52.4 32.6 50.2
North	: Site	Access												
7 9 Appro	L2 R2 bach	171 228 399	0.0 0.0 0.0	180 240 420	0.0 0.0 0.0	0.264 *0.684 0.684	27.1 44.1 36.8	LOS C LOS D LOS D	5.5 10.3 10.3	38.8 72.1 72.1	0.74 0.99 0.88	0.76 0.85 0.81	0.74 1.04 0.91	40.8 34.3 36.8
West	: Finuc	ane Roa	d											
10 11	L2 T1	190 1239	0.0 1.0	200 1304	0.0 1.0	0.143 * 0.695	8.8 18.2	LOS A LOS B	2.4 22.6	16.5 159.6	0.29 0.81	0.64 0.73	0.29 0.81	51.9 46.2
Appro All Vehic		1429 3541	0.9 1.2	1504 3727	0.9 1.2	0.695 0.695	17.0 16.7	LOS B	22.6 22.6	159.6 159.6	0.74 0.71	0.72 0.66	0.74 0.72	46.9 46.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance											
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of a Service	Level of AVERAGE BACK Service QUEUE [Ped Dist		Prop. Effective Que Stop Rate		Travel Time	Travel Dist. S	Aver. Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
East: Finucane	e Road										
P2 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	208.9	220.5	1.06
North: Site Acc	cess										
P3 Full	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	203.8	213.9	1.05
West: Finucan	e Road										
P4B ^{Slip/} Bypass	50	53	39.3	LOS D	0.1	0.1	0.94	0.94	196.4	204.3	1.04
All Pedestrians	150	158	39.3	LOS D	0.1	0.1	0.94	0.94	203.1	212.9	1.05

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. SIDRA INTERSECTION 9.0 | Copyright © 2000-2020 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: LAMBERT AND REHBEIN (SEQ) PTY LTD | Licence: PLUS / 1PC | Processed: Monday, 11 July 2022 12:02:56 PM Project: F:\Jobs\B19500\B19590\Design\Traffic\B19590 SIDRAS - Network.sip9

PHASING SUMMARY

Site: 101 [Finucane Road Site Access W/D 2034 PM (Site

Folder: Access SIDRAS)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Cycle Time)

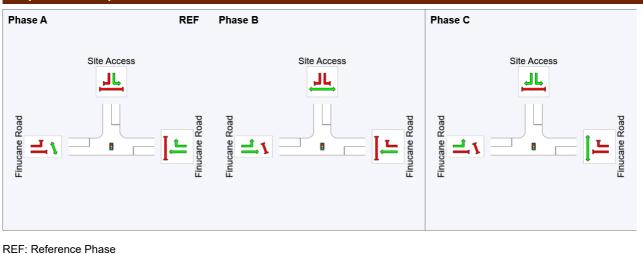
Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Leading Right Turn Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Summary

Phase	Α	В	С
Phase Change Time (sec)	0	16	67
Green Time (sec)	10	45	17
Phase Time (sec)	16	51	23
Phase Split	18%	57%	26%

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

Output Phase Sequence

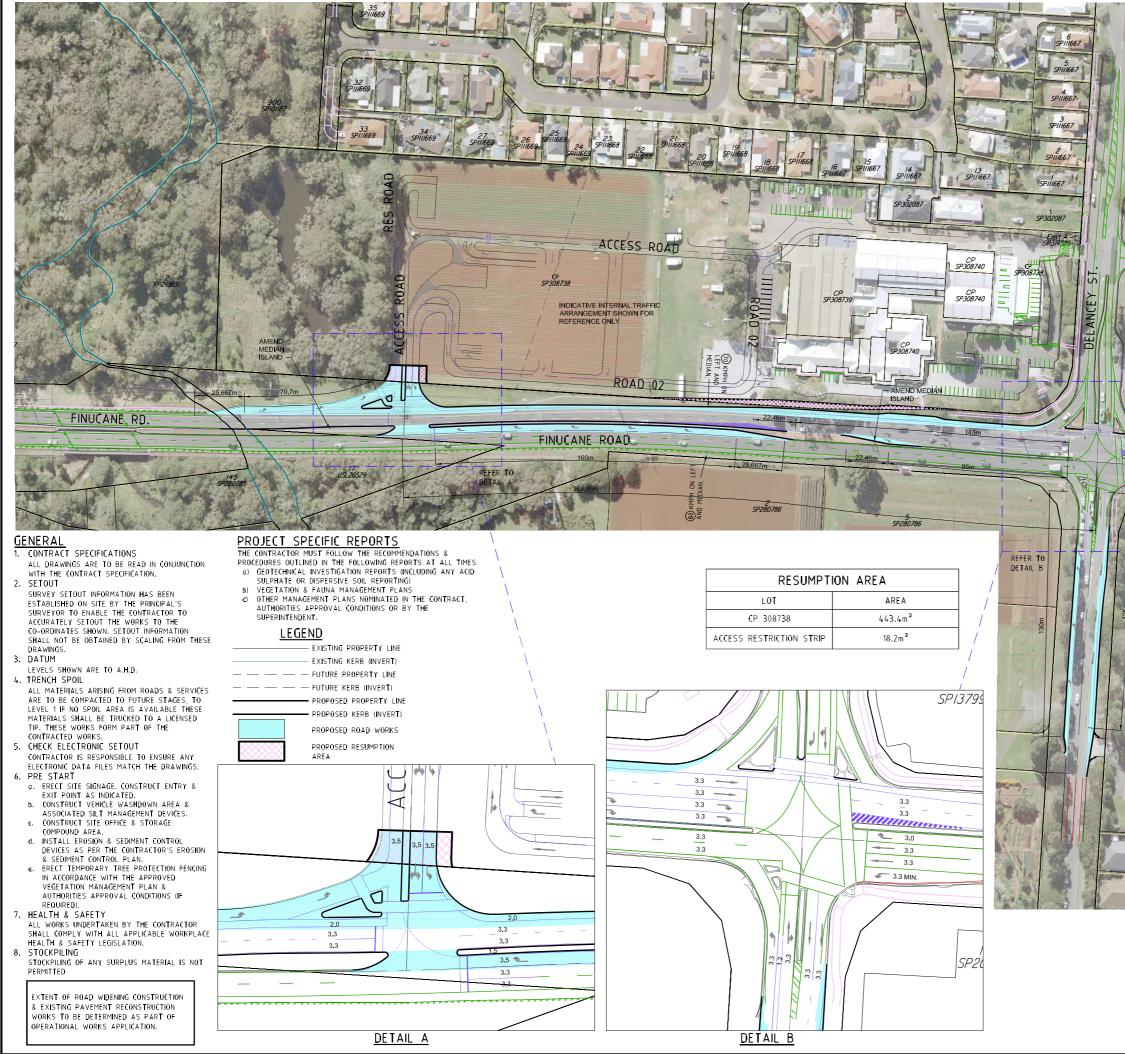


VAR: Variable Phase



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APPENDIX F- CONCEPT INTERSECTION DESIGNS





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